

THE DEVELOPMENT OF A LABORATORY  
TECHNIQUE FOR  
MODEL CONSTRUCTION

WILLIAM MANVILLE JOHNSON, JR.  
AND DANIEL NELSON SHOCKEY

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THE DEVELOPMENT OF A LABORATORY TECHNIQUE  
FOR MODEL CONSTRUCTION

Submitted to the Faculty of  
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By

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DANIEL NELSON SHOCKEY

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The following is a list of the equipment  
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the use of the testing equipment.

## INTRODUCTION

Present day structural design is based upon theory composed of numerous assumptions, some of which have been proved rigidly by experimental data while others have been shown to be adequate only so long as a large enough safety factor is introduced. The strength and stability of the majority of our structures which have been built in the last thirty years attest the overall adequacy of the theory being used. However, as stated above, this theory is padded in numerous places with high safety factors to insure adequacy in instances where experimental data is lacking. Of course, information which is lacking could be obtained by trial and error -- building a structure, loading it, and observing whether or not the structure supported the loads to which it was subjected. If a person lived long enough and had unlimited resources, he might obtain some very important information in this way. However, as has been done in the past and as will probably be done in the future, designers have attempted to make models of the structures they wished to investigate and, by subjecting those models to loads which simulated the actual loading, learn something about the action of the prototype. Model analysis has proved very useful in some instances.

In the field of rigid frames, for example, little is known about the stresses at the knees. Practically all the information we have at this time came from the results of some full and quarter scale tests conducted several years ago by the U. S. Bureau of Standards, Lehigh and Columbia Universities,

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and the University of Illinois. These tests were very expensive, and are not likely to be repeated for checking purposes in the near future. The results of these tests disagreed radically with the stresses predicted by theory. Although a new theory was evolved, to date it has not been checked. The small scale testing that has been done up to this time has not yielded, in general, satisfactory results.

In an effort to help solve this problem, E. J. Scullen designed and constructed at Rensselaer Polytechnic Institute in 1950, as a master's thesis, a model testing frame which could accommodate intermediate scale models (approximately one twenty-fifth to one fifteenth scale.) It was hoped that by testing intermediate scale models, accurate information could be obtained at much less expense than by testing large scale models.

The object of our thesis, then, was to develop a technique for constructing the intermediate scale models. The prime requirement of any technique would be to produce a model which could be expected to simulate the action of its prototype. The technique should be inexpensive. It should be simple, so that master craftsmen are not required to build the model. The technique should facilitate rapid construction of a model. Last of all, the technique should be flexible, lending itself to the fabrication of models of varied shapes.

In the attainment of the object as presented above, the authors constructed many different models and tested these models by various means to determine their suitability for model analysis.

to the fabrication of models of varied shapes. Last of all, the technique should be flexible, handy, itself. The technique should facilitate rapid construction of a model. That matter, discussed and not referred to in the model. The technique should be independent. It should be simple, so could be a section of a simulated section of the model. The element of any self-sufficient model is of itself. For constructing the model, the model should be able to be constructed of a variety of materials. The object of the model is to be able to be constructed of a variety of materials.

model analysis.

## I. CONSIDERATION OF MATERIALS TO BE USED

The problem of building a suitable model for laboratory analysis is a matter not only of the techniques and methods that might be used, but also a matter of what material should be used. Therefore, it is necessary first of all to look at the various materials readily available, and from these, to pick one or two that seem to possess the greatest possibilities for success.

Those materials which seemed to us to offer the best possibilities were: aluminum, steel, plastic, and wood.

An understanding of the problem of using the loading frame with the high loads which it will be desirable to apply will bring to mind a question about the feasibility of using wood and plastic. Wood, of course, is readily available, but the difficulty of fabricating suitable models such that reasonable values could be predicted for their prototypes is a major problem. Also, knowing that eventually it will be desirable to build welded structures, the making of suitable joints with wood that would resemble welded joints presents a problem of questionable solution. The possibility of using plastic is equally as difficult as using wood, not only because of the problem of putting joints together and the low loads plastics are capable of carrying, but of great importance is the fact that residual, stress free models are very difficult to make.

This then brings us to aluminum and steel. These two metals were chosen in preference to other metals due to the

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great deal of information that is known about them, such that the problem of making models might be simplified by using techniques already recognized as acceptable. It was decided to use aluminum first, primarily because of a ready supply on hand, along with the fact that the equipment available was best suited for handling that material. The most recognized characteristics of aluminum are its light weight, resistance to corrosion, and high strength, which make it highly desirable for this work. However, there are several properties of aluminum which tend to hinder the possibility of success. These are: (1) the fact that the melting point of aluminum is very close to the welding temperature such that great care is needed to avoid melting the parent material while welding, and; (2) the coefficient of thermal expansion of aluminum is slightly more than twice that of cast iron or steel with the resulting effect that care must be taken to consider expansion and to control it carefully in order to avoid distortion. Secondly, we decided to try steel as a material for a possible second method even if aluminum should work out. This would prove to be very helpful, if successful, since with the higher strength of steel it would be possible to build models which would be capable of carrying the full load of the loading frame.

Thus, with this in mind we started with aluminum as our first material and proceeded as the following pages indicate.

First material and proceeded as the following pages indicate. Thus, with this in mind we started with aluminum as our load of the loading frame.

## II. ALUMINUM MODULUS CHECK

In the fabrication of models from aluminum by brazing, soldering, or welding it is necessary to heat the aluminum, the temperature required depending upon the method used. Aluminum alloys which derive their strength from alloying and subsequent tempering\* are annealed by reheating (if the reheating temperature is high enough) and lose their strength. Aluminum alloys which derive their strength from alloying alone are not appreciably changed by heating them to temperatures below their melting points. Of the alloys tested, 61ST6 is one of the former, while 52SO is one of the latter. We were interested in finding out what happened to these alloys, with respect to their structural strength, specifically their moduli of elasticity, when they were heated to temperatures required for brazing, soldering, or welding. As stated in The Aluminum Company of America's literature, "Alcoa Aluminum and Its Alloys," and "Welding and Brazing Alcoa Aluminum," the results of heating these alloys could be determined for each case only by individual tests. We, therefore, elected to test various heated and unheated samples by using electric strain gage equipment.

### A. Electric Strain Gage Equipment\*\*

#### 1. General

The electric strain gage equipment used was

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\* Engineering Physical Metallurgy (Chapter 4) - Heyer

\*\* For a detailed description and for operation procedure, see Baldwin instruction book, bulletin 312, entitled "Type L Portable Strain Indicator."

\* For a detailed description and for operating instructions, see Baldwin Instrument Book, Bulletin 412, entitled "Type B Portable Strain Indicator."

The electric strain gage equipment used was

# I. General

## A. Electric Strain Gage Equipment

Using electric strain gage equipment.

For the purpose of this test, the electric strain gage equipment used was of the type known as "Type B Portable Strain Indicator." This equipment consists of a main unit and a separate power source. The main unit is a small, rectangular box containing the electronic circuitry and a meter. The power source is a battery pack that can be connected to the main unit. The equipment is designed to be used in a variety of ways, but for this test it was used to measure the strain in a metal specimen. The specimen was a cylindrical bar of metal, and the strain gage was attached to its surface. The equipment was connected to the specimen, and the strain was measured. The results of the test are shown in the following table.

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composed of, essentially, bonded wire strain gages, type SR-4, and an electric indicating device for measuring strains, in micro-inches, produced in those strain gages by some type of loading applied to the material upon which the gages were mounted. The indicating device, Baldwin Type 1, indicates strains resulting from the loading by measuring the change in electrical resistance produced in the bonded gages.

Leading to the indicator are two sets of wires, one set from the active gage and one set from the compensating gage. The active gage is mounted on the test piece or model which is to be loaded; the compensating gage is mounted on a piece of the same material as that on which the active gage is mounted, is placed near the active gage, but is not loaded. The purpose of the compensating gage is to correct the strain reading for temperature prevailing in the vicinity of the active gage.

## 2. Operating Procedure

- a. Check calibration of indicating device if equipment is being used for the first time.\*
- b. Check batteries; if the pointer remains in the red part of the dial, new batteries are needed.
- c. Connect leads from compensating and active gages to their respective terminals.

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\* See Calibration Check Procedure below.

active life.

S. 108607, p. 13, l. 9

[illegible]

... Connect them from competing and active  
... to their respective terminals.

\* See Calibration Check procedure below.

- d. Turn battery switch to ON position, and allow 10 seconds for tube warm-up.
- e. Set the correct value of gage factor, as supplied by the gage manufacturer, on the gage factor dial.
- f. Bring the pointer to zero, and read the indicator dial. This is the zero reading.
- g. Load the test piece, bring the pointer to zero, and read the indicator dial. This is the loaded reading. The difference between the zero reading and the loaded reading is the strain produced in the test piece by the load, in micro-inches.

For best results, use hot soldered joints in connecting lead wires to gages, and make both lead wires to any one gage the same length. Also, place the compensating gage as near as possible to the active gage.

### 3. Calibration Check

If the indicating equipment is being used for the first time, it is best to check its calibration before using it. A brief check procedure follows:

- a. Connect the active and compensating gages to the equipment as above, and set the gage factor dial.
- b. Take a zero reading.
- c. Connect a resistor of known value, ( $R_0$ ), in parallel with the active gage. A resistor of

1. Connect the active and compensating gages to the equipment as above, and set the gage factor dial.

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for each test, as has been suggested in the instructions, and wires to be as, and make both lead wires to any one, and the same length. Also, place the compensating gage as near as possible to the active gage.

### 3. Calibration Check

- If the indicating equipment is being used for the first time, it is best to check the calibration before using it. A check procedure follows:
- Connect the active and compensating gages to the equipment as above, and set the gage factor dial.
  - Take a zero reading.
  - Connect a resistor of known value, ( $R_0$ ), in parallel with the active gage. A resistor of

about 500,000 ohms is satisfactory.

d. Read the indicator dial, and subtract the zero reading from this second reading to obtain a value in micro-inches which we will call "e".

e. If  $R_g$  designates the resistance of the active gage, which is approximately 120 ohms, then  $R_c$  referred to in "c" above equals  $R_g$  divided by e times the gage factor, G.

$$\text{ie, } R_c = \frac{R_g}{(e)(G)}$$

This computed value of  $R_c$  should equal the value of the known resistor.

#### B. Preparation of Samples

Strips of 61ST6 aluminum about 9 inches long and about 1 inch wide were cut from sheet aluminum 0.091 inches thick. The cutting was done on a metal cutting bandsaw. The edges of the pieces were sanded to remove cutting burrs. Similar strips 0.271 inches thick were cut from 5280 stock. Two samples each of 61ST6 and 5280 were then heated with an oxy-acetylene torch, with an effort being made to simulate the welding and soldering temperatures. As an index to the correct temperature, the pieces were heated until the flame impinging upon the aluminum became tinted with yellow. This was an arbitrary temperature measuring index (which later proved inaccurate) adopted after observing the flame while actually joining pieces of aluminum. The heated pieces were then air cooled.



Another strip of 61ST6 was heated in an electric furnace to the actual temperature required for welding, then allowed to cool in air.

An electric strain gage of the SR4 type was then mounted on the centerline of each piece at about its mid-length.

C. Explanation of the Gage Mounting Procedure

1. Clean the surface upon which the gage is to be mounted. For this purpose, light grinding or sanding with emery cloth may be employed.

2. Degrease the surface with carbon tetrachloride (acetone may be used).

3. Mount the gage:

- a. Scribe lines to indicate gage orientation.
- b. Coat test surface with a layer of Duco household cement and allow it to dry about 20 minutes.
- c. Coat test surface with a second liberal coat of Duco cement and allow it to dry until it becomes slightly tacky.
- d. Press gage into position with proper orientation and gradually press out the excess cement with the fingers. Watch the corners of the gage particularly.
- e. Keep a slight pressure on the gage until the cement will hold the gage to the surface (about 3 minutes required).
- f. Cure the gage:

(1) Cure gages under a slight pressure - about

(1) Cure tapes under a slight pressure - about

1. Cure the tape:

3 minutes required).

cement will hold the tape to the surface (about  
e. Keep a slight pressure on the tape until the

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the and gradually press out the excess cement with

2. Press the tape into position with proper pressure.

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c. Now this surface is a second liberal coat

of the cement to allow it to dry until it has

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4. Now apply a fourth coat of cement to the upper

(Note: The above steps are for the upper

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8. Now apply an eighth coat of cement to the upper

9. Now apply a ninth coat of cement to the upper

10. Now apply a tenth coat of cement to the upper

11. Now apply an eleventh coat of cement to the upper

12. Now apply a twelfth coat of cement to the upper

13. Now apply a thirteenth coat of cement to the upper



1 pound will be sufficient.

(2) Directly on top of the gage, place a layer of waxed paper, then a piece of sponge rubber, then the weight. This combination allows the slight pressure of the weight to hold the gage in place while curing.

(3) Allow gages to cure at room temperature for at least 24 hours. If curing is taking place in an atmosphere of high humidity, allow a longer curing time.

(4) As an alternative to (3) above, an infra-red heating bulb may be placed near the gages, such that a temperature of 150° is maintained, in which case only about 5 hours curing time is required.

4. Cover gages with a light coating of Ceresin wax to keep out moisture. (If testing is being conducted in a laboratory, in all probability no wax coating will be required.)

#### D. Check of Mounted Gages

After gages have been cured, it is necessary to check them before straining them. The resistance of a strain gage should be about 120 ohms. The leakage resistance to ground should be infinite. By using an ohmmeter, check the above resistances. The gage resistance, in order to be satisfactory, should be within 2 ohms of 120 ohms. The resistance to ground should be at least 50 megohms.

1. The first test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

2. The second test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

3. The third test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

4. The fourth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

5. The fifth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

6. The sixth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

7. The seventh test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

8. The eighth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

9. The ninth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

10. The tenth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

11. The eleventh test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

12. The twelfth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

13. The thirteenth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

14. The fourteenth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

15. The fifteenth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

16. The sixteenth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

17. The seventeenth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

18. The eighteenth test is to check the resistance of the circuit. This is done by connecting a voltmeter across the terminals of the circuit and measuring the voltage. The voltage should be about 180 ohms. The resistance should be infinite. If the resistance is not infinite, the circuit is faulty and should be replaced.

to keep out moisture. (If the circuit is faulty, it should be replaced.) Laboratory, in all probability no wax coating, will be required.)

D. Check of Mounted Pages

After pages have been cured, it is necessary to check them before straining them. The resistance of a strain, page should be about 180 ohms. The leakage resistance to ground should be infinite. By using an ohmmeter, check the above resistances. The page resistance, in order to be satisfactory, should be within 8 ohms of 180 ohms. The resistance to ground should be at least 50 megohms.

E. Testing of Samples and Results (See Figures 1 and 2)

The samples were loaded as cantilevers, one end being held with a "C" clamp to a rigid support while the other end received the load.

Loading was accomplished by suspending an empty beer can, into which shot was placed, from a knife edge which rested in a deep scribe mark at the end of the test piece. Loads were varied by varying the amount of shot placed in the can. The shot was weighed on a laboratory balance for accuracy.

Representative results of these tests are shown on the next few pages.

1. The first of these is the fact that the

the second is the fact that the

the third is the fact that the

received the same

the fourth is the fact that the

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the sixth is the fact that the

the seventh is the fact that the

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the ninth is the fact that the

the next row

## Modulus Check

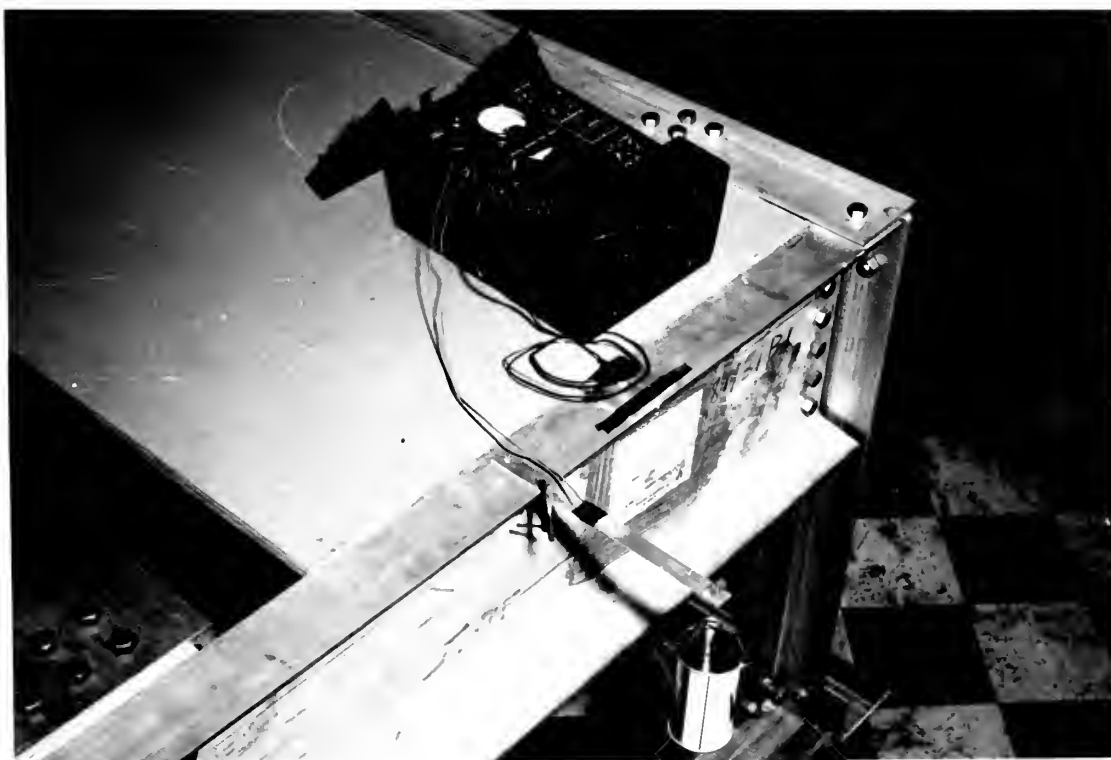


Figure 1



## Method of Loading For "E" Check

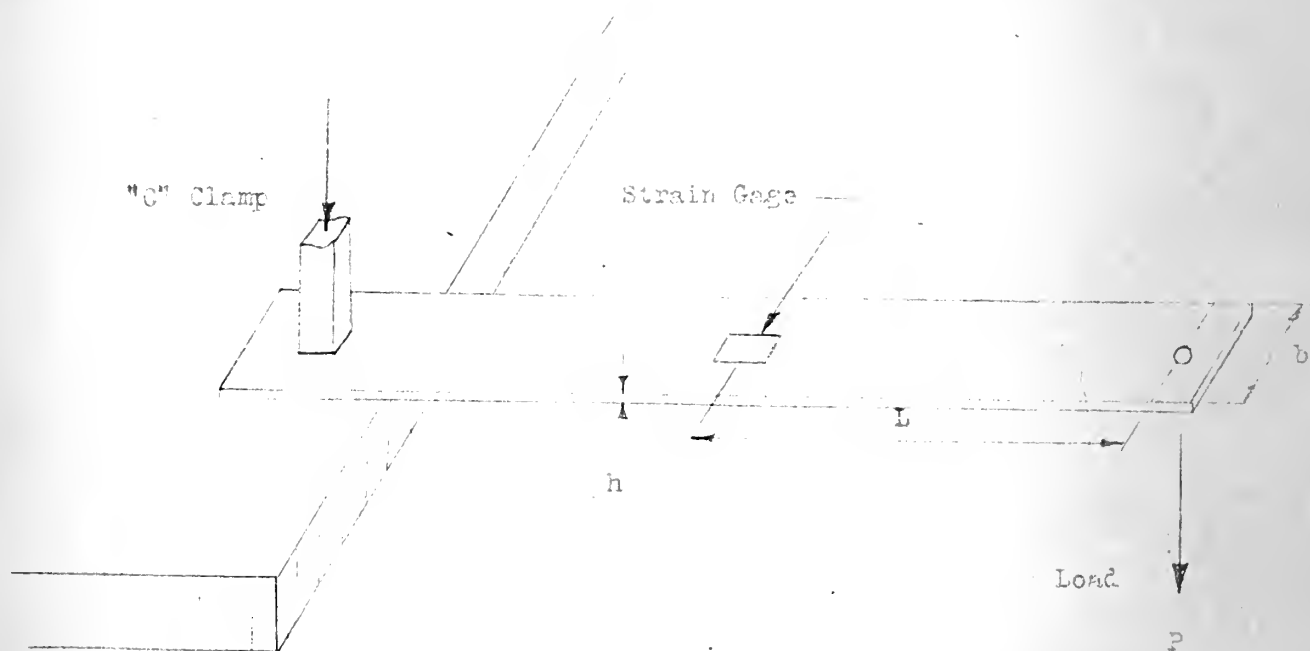


Figure 2





## Form of Computations for "E" Check

## Computations:

$$I = \frac{bh^3}{12}$$

$$f = \frac{Mc}{I} = \frac{(P)(L)(h/2)}{\left(\frac{bh^3}{12}\right)} = \frac{6 PL}{bh^2}$$

$$E = K \frac{\text{Stress}}{\text{Strain}} = K \frac{f}{e} = \frac{6 PL}{6 \frac{h^2}{e}} K$$

## Terms Defined:

I, moment of inertia, inches<sup>4</sup>

b, width of test piece, inches

h, thickness of test piece, inches

f, bending stress in extreme fiber of test piece, lbs/inch<sup>2</sup>

e, unit strain indicated by SR-4 gage, micro-inches/inch

E, a number proportional to the modulus of elasticity

L, distance from point of application of load to the center of the strain gage, inches

M, bending moment, inch lbs.

K, a constant which corrects the strain indicated by the SR-4 gage to the value actually existing at the extreme fiber of the test piece.

P, load applied, pounds

1. 1. 1.

2.

3. 1. 1. 1.

4. 1. 1. 1.

5. 1. 1. 1. 1.

6. 1. 1. 1. 1. 1.

7. 1. 1. 1. 1. 1. 1.

8. 1. 1. 1. 1. 1. 1. 1.

9. 1. 1. 1. 1. 1. 1. 1. 1.

10. 1. 1. 1. 1. 1. 1. 1. 1. 1.

11. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.

12. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.

13. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.

14. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.

15. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.

## Heated 618T6 Aluminum

## Dimensions of Test piece:

$$b = 0.817 \text{ inches}$$

$$I = 5.21 \times 10^{-5} \text{ inches}^4$$

$$h = 0.091 \text{ inch}$$

$$L = 5.34 \text{ inches}$$

$$E = \frac{P}{e} (.00466)$$

| P, lbs. | Strain Reading |        |     |      |      |
|---------|----------------|--------|-----|------|------|
|         | Zero           | Loaded | e   | E    | f    |
| .25     | 0924           | 1043   | 119 | 9.78 | 1165 |
| .50     | 0924           | 1161   | 237 | 9.82 | 2330 |
| .75     | 0923           | 1281   | 358 | 9.78 | 3490 |
| 1.00    | 0923           | 1400   | 477 | 9.78 | 4660 |
| *1.25   | 0923           | 1519   | 596 | 9.78 | 5840 |
| *1.50   | 0923           | 1637   | 714 | 9.80 | 7000 |

\* Strains recorded are those existing 10 minutes after the load was applied. Immediately upon applying the load, the strain was somewhat higher, but gradually decreased to the above values. After 10 minutes, there was no significant change in the strain reading.

1. The first two columns are the same as in the previous table.

2. The third column is the same as in the previous table.

3. The fourth column is the same as in the previous table.

4. The fifth column is the same as in the previous table.

5. The sixth column is the same as in the previous table.

6. The seventh column is the same as in the previous table.

| 1   | 2   | 3   | 4   | 5   | 6   |
|-----|-----|-----|-----|-----|-----|
| 1.1 | 1.1 | 1.1 | 1.1 | 1.1 | 1.1 |
| 1.2 | 1.2 | 1.2 | 1.2 | 1.2 | 1.2 |
| 1.3 | 1.3 | 1.3 | 1.3 | 1.3 | 1.3 |
| 1.4 | 1.4 | 1.4 | 1.4 | 1.4 | 1.4 |
| 1.5 | 1.5 | 1.5 | 1.5 | 1.5 | 1.5 |
| 1.6 | 1.6 | 1.6 | 1.6 | 1.6 | 1.6 |
| 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| 1.8 | 1.8 | 1.8 | 1.8 | 1.8 | 1.8 |
| 1.9 | 1.9 | 1.9 | 1.9 | 1.9 | 1.9 |
| 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |

\* Strain recorded and then existing in the material after the load was applied. Immediately upon applying the load, the strain was somewhat higher, but gradually decreased to the above values. After 10 minutes, there was no significant change in the strain reading.

## Unheated 61ST6 Aluminum

Dimensions of test piece:

$$b = 0.837 \text{ inches}$$

$$I = 5.33 \times 10^{-5} \text{ inches}^4$$

$$h = 0.0915 \text{ inches}$$

$$L = 5.375 \text{ inches}$$

$$E = \frac{P}{\delta} (.00457)$$

## Strain Reading

| <u>P, lbs.</u> | <u>Zero</u> | <u>Loaded</u> | <u>e</u> | <u>E</u> | <u>f</u> |
|----------------|-------------|---------------|----------|----------|----------|
| .25            | 1498        | 1617          | 119      | 9.60     | 1143     |
| .50            | 1497        | 1735          | 238      | 9.60     | 2281     |
| .75            | 1496        | 1853          | 357      | 9.60     | 3430     |
| 1.00           | 1494        | 1970          | 476      | 9.60     | 4570     |
| *1.25          | 1493        | 2086          | 593      | 9.62     | 5700     |
| *1.50          | 1493        | 2201          | 708      | 9.68     | 6860     |

- \* Strains recorded are those existing 10 minutes after the load was applied. Immediately upon applying the load, the strain was somewhat higher, but gradually decreased to the above values. After 10 minutes, there was no significant change in the strain reading.



## P. Conclusions on "E" Check

As shown on the preceding pages, the torch heating of the 61ST6 strips changed their value of "E" about 2%. Consistent, stable strain readings were obtained as long as the stresses were below about 5000 psi. For stresses above about 5000 psi, strains fluctuated with time.

Stressing of the 52SO strips, heated and unheated, produced strains which varied radically with time, even at low values of stress. As shown on figures 3 and 4, this variation appears almost linear on a semi-log graph plot. This action appears similar to creep, only in a reversed direction, the test piece seeming to gain strength (i.e., become strained less) the longer the load of constant value remains on it. The authors consulted with members of the Metallurgy Department in an effort to explain this action, but were unable to find a satisfactory answer.

The authors concluded from the results of the above tests that 52SO definitely would not be suitable for a model material, but that 61ST6 would probably be satisfactory.

In an effort to determine the cause of poor results in beams #1 through #7, strips of 61ST6 were placed in an electric furnace to find out the actual temperature required for fusion. It was determined that 1125° F. was required for fusion of the parent metal with the eutecrod filler. The temperature left the metal in a very soft distorted condition after being heated for about 15 minutes. When an effort was made to subject a strip to a flexural load, it collapsed.

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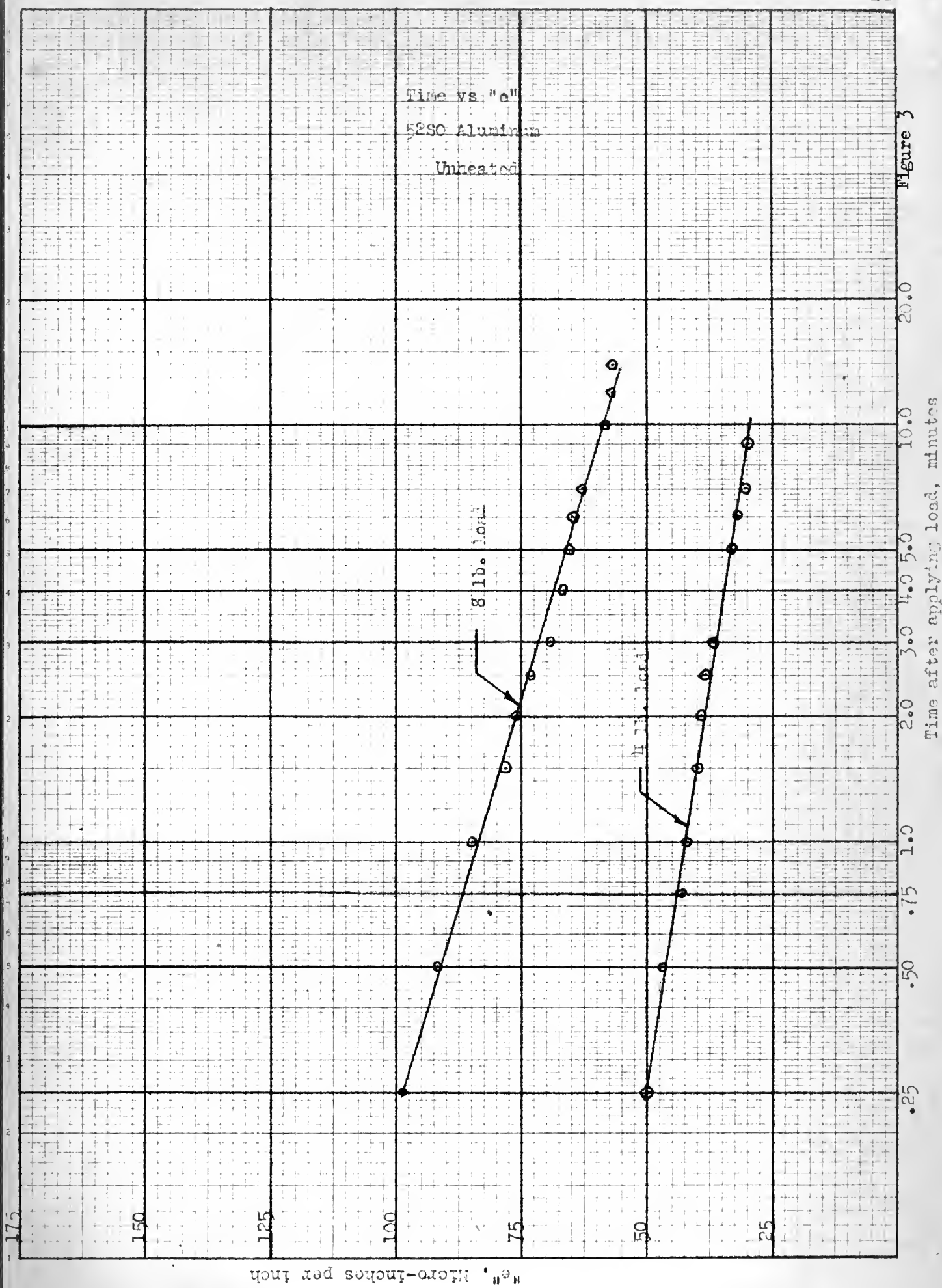


Figure 3



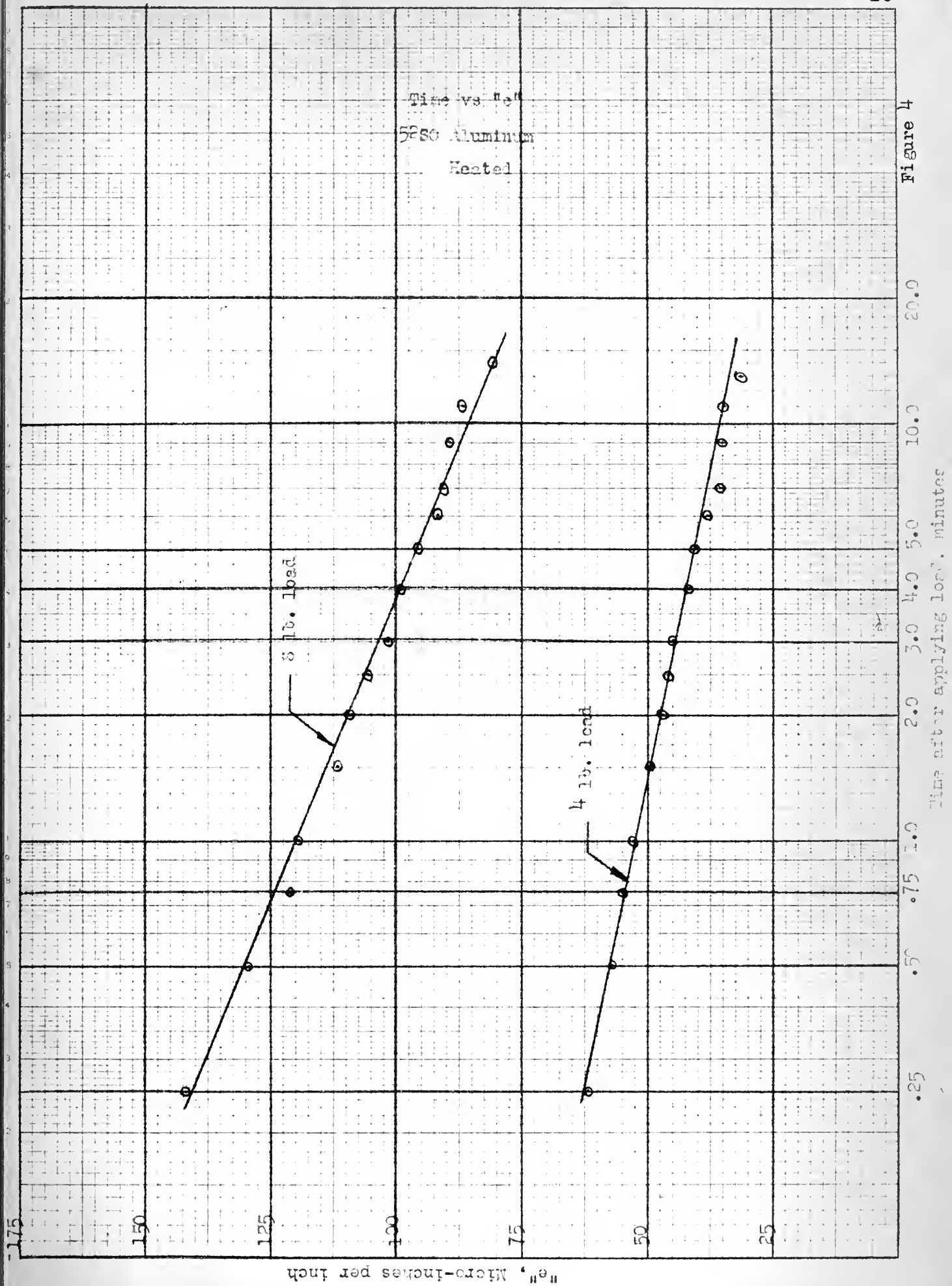


Figure 4



Thus, it was impossible to get strain reading with any meaning from these pieces. The authors concluded that, since these strips had been rendered useless for structural purposes by the temperature required for fusion, at least in the vicinity of the weld when a torch was being used to supply the heat, a similar condition of softness and distortion existed. It was seen, then, that the amount of torch heating given the 61ST6 strips, as determined arbitrarily by the slight discoloration of the flame as it impinged upon the surface of the aluminum, was in reality considerably below the temperature required for fusion during welding with eutecrod. This accounts for the closeness of the valves of "E" as determined in the previous test. (The actual temperature was probably near that required for soldering with the alladin rod.) Welding was then discarded as a method of making aluminum models, and our efforts were concentrated upon the lower temperature alladin soldering method.

U

B

S

A

C

W

1881

12

1881

1881

1881

1881

1881

1881

### III. FABRICATION OF TEST BEAMS

The two materials used for the making of the test beams were steel and aluminum, with the reasons for this choice as dictated in part I. This section is dedicated to the methods and techniques used, and the problems encountered in the fabrication of the test beams. It is divided into two sub parts based on the material used.

#### A. Aluminum

Aluminum was our first choice for the reasons previously explained. It is best perhaps to begin with the ways the material was prepared.

##### 1. Preparation of Material

The aluminum for the beams was obtained from sheet aluminum of the 61ST6 type, and of varying thicknesses. A metal cutting bandsaw was used to obtain the desired sizes, with great care being taken to insure that straight pieces were obtained. Of interest at this point, would be the fact that it is necessary to be certain that a sharp blade is used in the saw if a straight cut is to be obtained. The sawed edges were next ground down to a smooth finish on a disc sand-wheel and burrs left from this sanding were taken off with a belt sander. This process insured that the extreme edge was not only clean, but also smooth such that close fitting tolerances were obtained when joining pieces. In the making of wide-flange beams in which the web of the beam is butted against the middle of the flange, it is felt by the authors

... were about ...  
... and ...  
... based ...

1. ...

... explosion ...  
... material ...

1. ...

... sheet aluminum of the 1110 type ...  
A metal cutting process ...  
with great care being taken ...  
were obtained. Of course ...  
that it is necessary to be certain ...  
in the saw if a straight cut is to be obtained. The sawed  
edges were next ground down to a smooth finish on a lathe sand-  
wheel and parts left from this sanding were taken off with a  
belt sander. This process insured that the extreme ends were  
not only clean, but also smooth and free from burrs. In the making of  
flanges were obtained when joining pieces. In the making of  
wide-flange beams in which the web of the beam is bolted  
against the middle of the flange, it is felt by the authors



that even the thin aluminum oxide present at the joint should be cleaned off. This was accomplished by using a fine emery cloth and cleaning the center of the flange along which the web would touch. The surfaces of the web near the edges were also cleaned up for a short distance, using emery cloth, to insure that the fillet of joining material would have a good surface on which to adhere.

## 2. Jigs

Making a jig to hold the pieces together while joining them was one of the most difficult problems encountered. We will explain not only the most successful method used, but also the others that were tried. It can be easily understood that the problem of jiggling is not just one of holding the materials, but also a problem of holding them extremely accurately in their correct relation to each other. For example, in the making of wide-flange beams it is necessary that the web be held exactly in the center of the flange. The problem of jiggling is applicable to all the different methods of joining the materials; therefore, it is only necessary to present it once.

At the beginning, the most important problem of jiggling seemed to be one of being able to insure that the pieces were held in exact alignment. It was with this in mind that the first jig was made.

This jig was constructed using three pieces of aluminum angle, lined with asbestos along the outside against



which the main material would be held, as shown in Figure 5. Angle A and angle B were clamped together to hold the flange securely. Then angle C, which was a cut down angle to give the maximum torch clearance, was used in conjunction with angle A to hold the web securely. As can be seen in the sketch, this method not only gave assurance that the web and the flange were at right angles, but also afforded clear access for measurement to insure that the web was at the midpoint of the flange. The several disadvantages that became apparent in using this method were as follows: (1) in spite of the asbestos lining, too much heat was lost through contact with the metal jig, such that the heating of the piece was irregular and thus the welding temperature, which is very critical, was hard to regulate; (2) the two pieces which were to be joined were both held clamped together, and although of the same material, they warped because of the unequal expansion due to localized heating; and (3) most important, as there was no support for the upper part of the flange, there was a tendency for it to distort to one side or the other due to the concentrated heating near the centerline. Therefore, in the method of welding using eutecrod, as will be explained later, the temperatures required are too high; however, in the soldering method, with the slightly lower temperatures, it might be possible to use the above procedure. The method finally used, as will be explained, seems to be a much more practical way of solving the problem.



Jig # 1

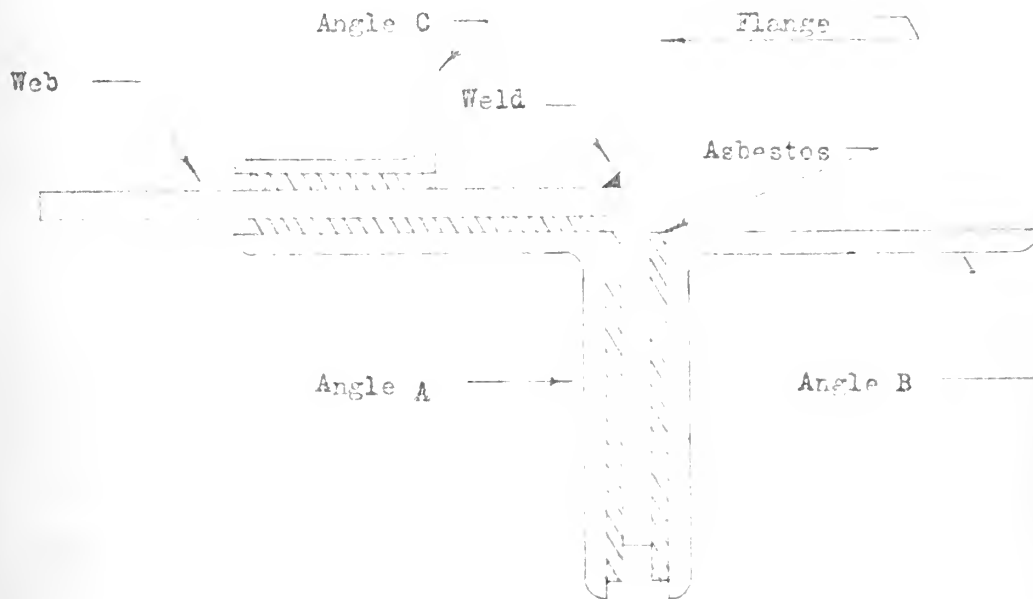


Figure 5



The second jig was made with the idea in mind of being able to support both flanges and the web at the same time to overcome any tendency of these members to warp due to lack of support. We then constructed a jig, the cross section of which is shown in Figure 6. The two angles were made about 30 inches long, which of course limited the length of beam it was possible to make. The angles were lined with asbestos and one was fixed to a base plate to prevent movement. The other angle was made a sliding variety which was held in place by clamping it to the fixed angle with "C" clamps to provide the pressure needed to hold the beam while welding it. The correct location of the web was obtained by using a piece of sheet aluminum bent on a brake such that it held the web up between the two flanges as shown in the sketch. In the process of welding, the two upper welds were placed, the beam was turned over, and the two other welds were placed. This method, at first, seemed to be the solution to all jiggling problems; however, one of the problems encountered in the first jig was present along with a new one. The old problem was that of controlling the heat, and still hadn't been solved. The new problem was as follows. In clamping the two angles together we tried to put just enough pressure to cause the joints to be tight, but not really forced together. This appeared fine from the standpoint of expansion, but still proved inadequate. From the sketch it can be seen that there is no easy way to provide support on top of the web, and still leave room in





Jig # 2

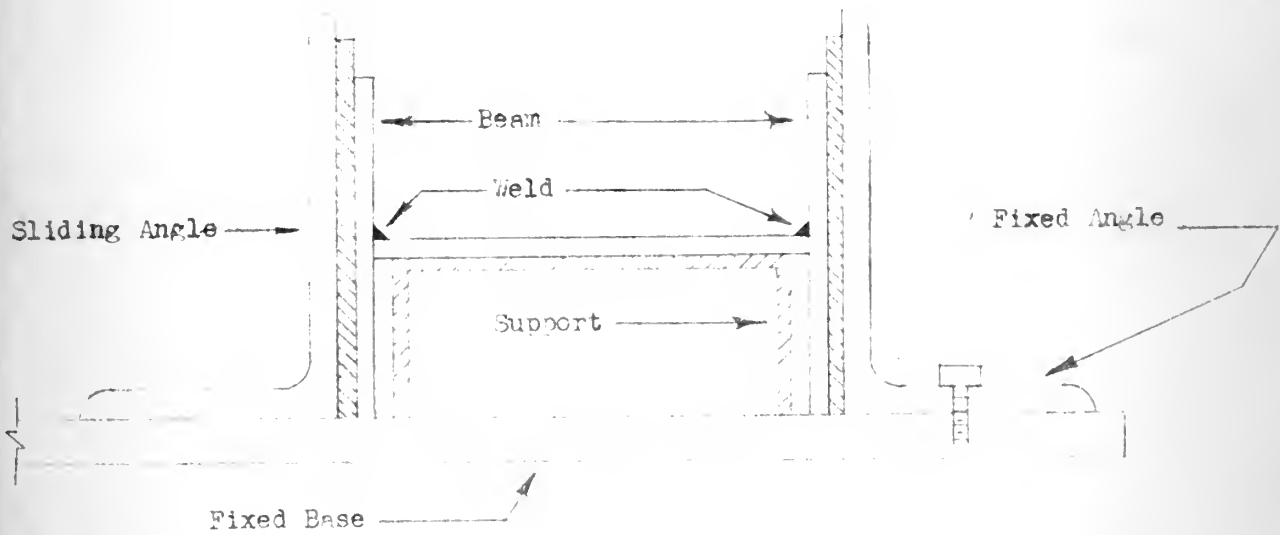


Figure 6



which to use the acetylene torch. Therefore, due to no top support, and the fact that the web was bound to be held by the flanges with more pressure in some places than others, there was a definite tendency for the web to rise off its support and buckle upwards when the heat was applied. The flanges seemed to stay in line, but the method resulted in a beam whose web was not exactly centered between the flanges. Therefore, this beam could not be expected to check according to the deflection theory being used.

Our third design, which eventually led to our final and very successful method, came as an effort to eliminate the defects that were noted in our previous jigs. First of all we wanted to eliminate the heat loss due to contact of other materials with those we were welding. It was also necessary to find some way to support the flanges and web such that they would be held in the proper orientation with respect to each other. These problems were solved by using 1-inch by 1-inch steel angle cut in 3/4-inch lengths. The flange and the web were held at right angles by clamping pieces of the angle to both the web and the flange along one side leaving the other free for welding. Then by adjusting the location of the angle on the flange the web could be placed in the proper location. This arrangement of angles was made along the whole length of the beam, the weld being placed down the free side. However, the flange and the web, which were clamped rigidly together, distorted due to the heating. This resulted in beams which would not check out.



Finally, we used the same method as described in the previous paragraph, but clamped one piece of angle to each side of the flange, directly opposite each other, leaving enough clearance between the angles to insert the web and hold it firm and perpendicular. (See Figures 7 and 8.) The pairs of angles were placed about one foot apart.

A clamp preventing the web and flange from separating, but allowing longitudinal movement, was placed along the beam at each set of angles. The whole beam was then supported on pedestals placed at the mid-point between the angles. This was done such that the beam reaction at the support would keep the joint between the web and the flange tight. The welding was done next, welding first on one side of the web for a length of about 8 inches, then on the other side of the web. It is important not to weld any closer than 2 inches to the angles. The purpose of welding on the opposite side immediately was to utilize the heat that had already been put into the pieces. This also minimized distortion, since stresses resulting from heating on both sides of the web tended to be cancelled out. After the whole beam was welded this way, it was necessary to take off the angles and clamps and weld up the remaining spaces. This method gave us consistent results on all beams constructed, as the results in the following sections will indicate.

### 3. Check of the Loading Device

It was felt by the authors that a check of the vertical loading frame (see Figure 9) was necessary in order

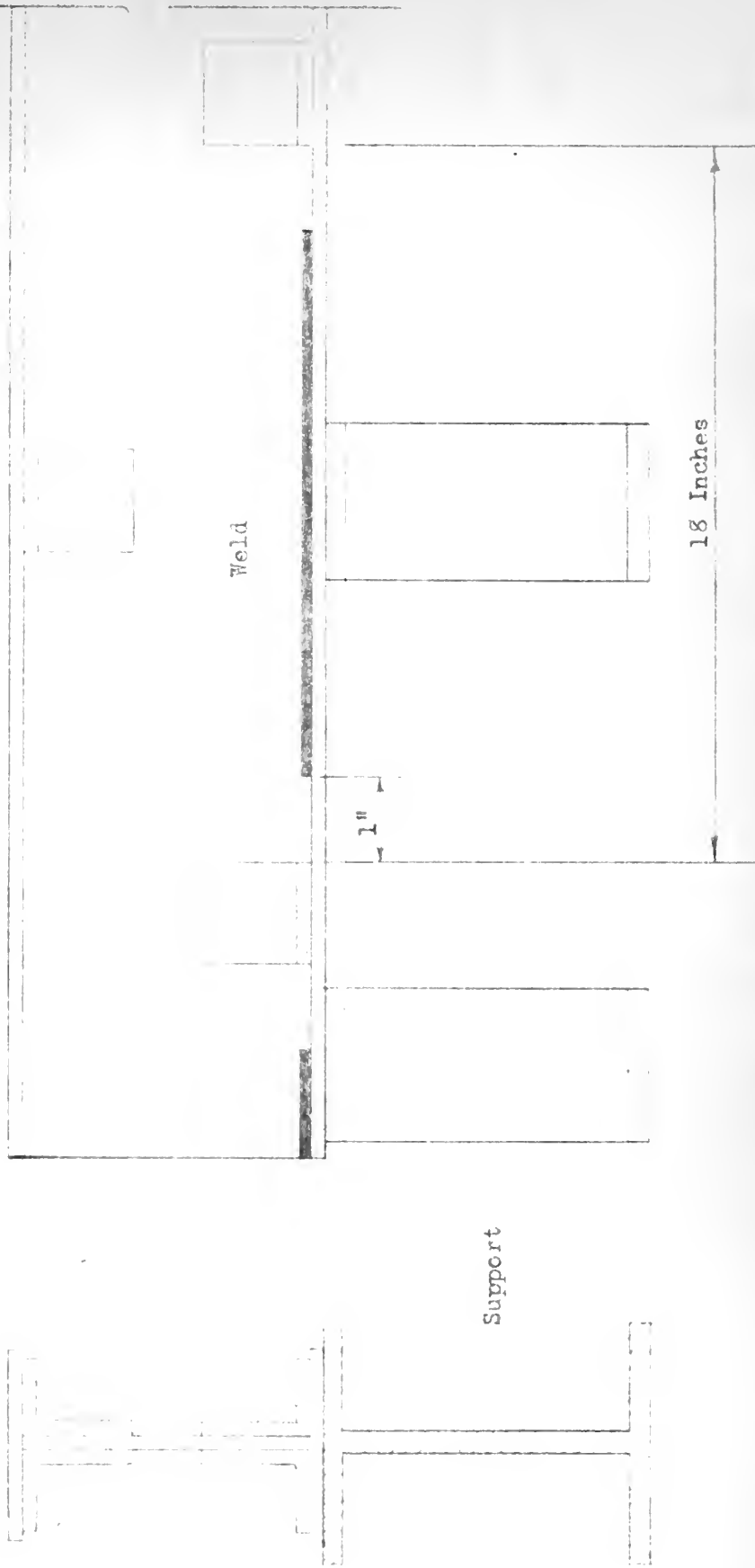
It was held by the majority that a finding of the  
Verdict should be set aside (see also 100, 101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 114, 115, 116, 117, 118, 119, 120, 121, 122, 123, 124, 125, 126, 127, 128, 129, 130, 131, 132, 133, 134, 135, 136, 137, 138, 139, 140, 141, 142, 143, 144, 145, 146, 147, 148, 149, 150, 151, 152, 153, 154, 155, 156, 157, 158, 159, 160, 161, 162, 163, 164, 165, 166, 167, 168, 169, 170, 171, 172, 173, 174, 175, 176, 177, 178, 179, 180, 181, 182, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 197, 198, 199, 200, 201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215, 216, 217, 218, 219, 220, 221, 222, 223, 224, 225, 226, 227, 228, 229, 230, 231, 232, 233, 234, 235, 236, 237, 238, 239, 240, 241, 242, 243, 244, 245, 246, 247, 248, 249, 250, 251, 252, 253, 254, 255, 256, 257, 258, 259, 260, 261, 262, 263, 264, 265, 266, 267, 268, 269, 270, 271, 272, 273, 274, 275, 276, 277, 278, 279, 280, 281, 282, 283, 284, 285, 286, 287, 288, 289, 290, 291, 292, 293, 294, 295, 296, 297, 298, 299, 300, 301, 302, 303, 304, 305, 306, 307, 308, 309, 310, 311, 312, 313, 314, 315, 316, 317, 318, 319, 320, 321, 322, 323, 324, 325, 326, 327, 328, 329, 330, 331, 332, 333, 334, 335, 336, 337, 338, 339, 340, 341, 342, 343, 344, 345, 346, 347, 348, 349, 350, 351, 352, 353, 354, 355, 356, 357, 358, 359, 360, 361, 362, 363, 364, 365, 366, 367, 368, 369, 370, 371, 372, 373, 374, 375, 376, 377, 378, 379, 380, 381, 382, 383, 384, 385, 386, 387, 388, 389, 390, 391, 392, 393, 394, 395, 396, 397, 398, 399, 400, 401, 402, 403, 404, 405, 406, 407, 408, 409, 410, 411, 412, 413, 414, 415, 416, 417, 418, 419, 420, 421, 422, 423, 424, 425, 426, 427, 428, 429, 430, 431, 432, 433, 434, 435, 436, 437, 438, 439, 440, 441, 442, 443, 444, 445, 446, 447, 448, 449, 450, 451, 452, 453, 454, 455, 456, 457, 458, 459, 460, 461, 462, 463, 464, 465, 466, 467, 468, 469, 470, 471, 472, 473, 474, 475, 476, 477, 478, 479, 480, 481, 482, 483, 484, 485, 486, 487, 488, 489, 490, 491, 492, 493, 494, 495, 496, 497, 498, 499, 500, 501, 502, 503, 504, 505, 506, 507, 508, 509, 510, 511, 512, 513, 514, 515, 516, 517, 518, 519, 520, 521, 522, 523, 524, 525, 526, 527, 528, 529, 530, 531, 532, 533, 534, 535, 536, 537, 538, 539, 540, 541, 542, 543, 544, 545, 546, 547, 548, 549, 550, 551, 552, 553, 554, 555, 556, 557, 558, 559, 560, 561, 562, 563, 564, 565, 566, 567, 568, 569, 570, 571, 572, 573, 574, 575, 576, 577, 578, 579, 580, 581, 582, 583, 584, 585, 586, 587, 588, 589, 590, 591, 592, 593, 594, 595, 596, 597, 598, 599, 600, 601, 602, 603, 604, 605, 606, 607, 608, 609, 610, 611, 612, 613, 614, 615, 616, 617, 618, 619, 620, 621, 622, 623, 624, 625, 626, 627, 628, 629, 630, 631, 632, 633, 634, 635, 636, 637, 638, 639, 640, 641, 642, 643, 644, 645, 646, 647, 648, 649, 650, 651, 652, 653, 654, 655, 656, 657, 658, 659, 660, 661, 662, 663, 664, 665, 666, 667, 668, 669, 670, 671, 672, 673, 674, 675, 676, 677, 678, 679, 680, 681, 682, 683, 684, 685, 686, 687, 688, 689, 690, 691, 692, 693, 694, 695, 696, 697, 698, 699, 700, 701, 702, 703, 704, 705, 706, 707, 708, 709, 710, 711, 712, 713, 714, 715, 716, 717, 718, 719, 720, 721, 722, 723, 724, 725, 726, 727, 728, 729, 730, 731, 732, 733, 734, 735, 736, 737, 738, 739, 740, 741, 742, 743, 744, 745, 746, 747, 748, 749, 750, 751, 752, 753, 754, 755, 756, 757, 758, 759, 760, 761, 762, 763, 764, 765, 766, 767, 768, 769, 770, 771, 772, 773, 774, 775, 776, 777, 778, 779, 780, 781, 782, 783, 784, 785, 786, 787, 788, 789, 790, 791, 792, 793, 794, 795, 796, 797, 798, 799, 800, 801, 802, 803, 804, 805, 806, 807, 808, 809, 810, 811, 812, 813, 814, 815, 816, 817, 818, 819, 820, 821, 822, 823, 824, 825, 826, 827, 828, 829, 830, 831, 832, 833, 834, 835, 836, 837, 838, 839, 840, 841, 842, 843, 844, 845, 846, 847, 848, 849, 850, 851, 852, 853, 854, 855, 856, 857, 858, 859, 860, 861, 862, 863, 864, 865, 866, 867, 868, 869, 870, 871, 872, 873, 874, 875, 876, 877, 878, 879, 880, 881, 882, 883, 884, 885, 886, 887, 888, 889, 890, 891, 892, 893, 894, 895, 896, 897, 898, 899, 900, 901, 902, 903, 904, 905, 906, 907, 908, 909, 910, 911, 912, 913, 9

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Angles Held With "C" Clamps

Beam



Jig # 3 Modified

Figure 7





Jig #3 Modified

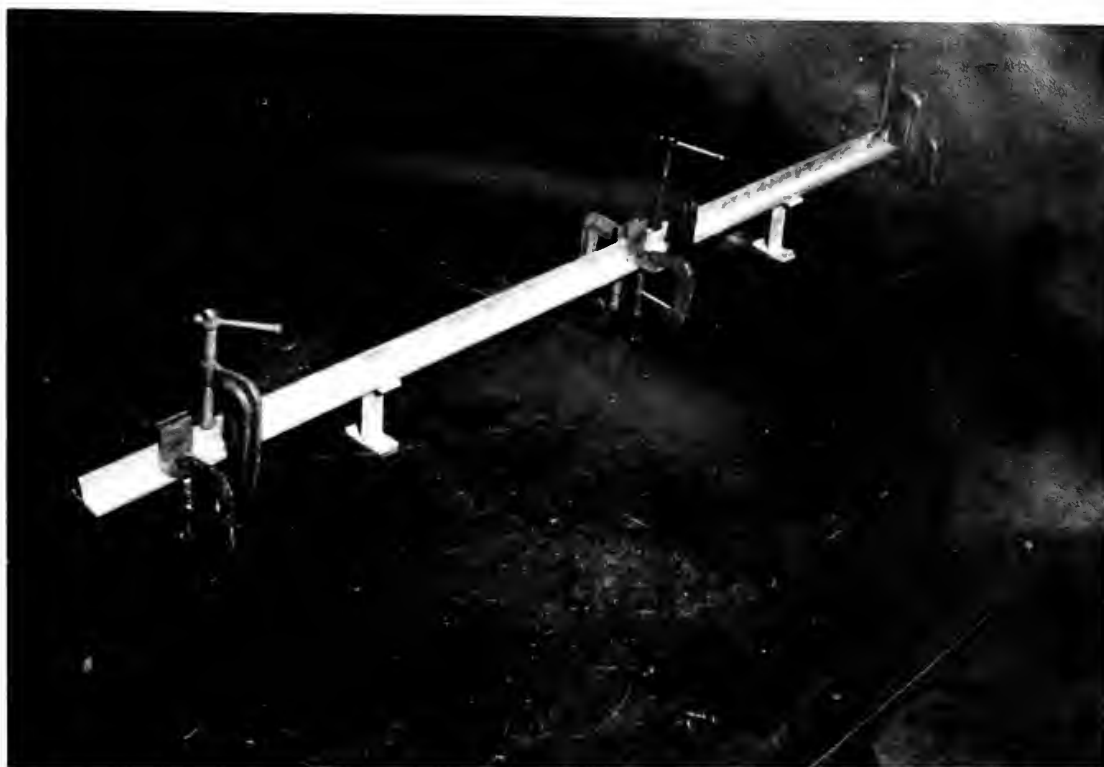
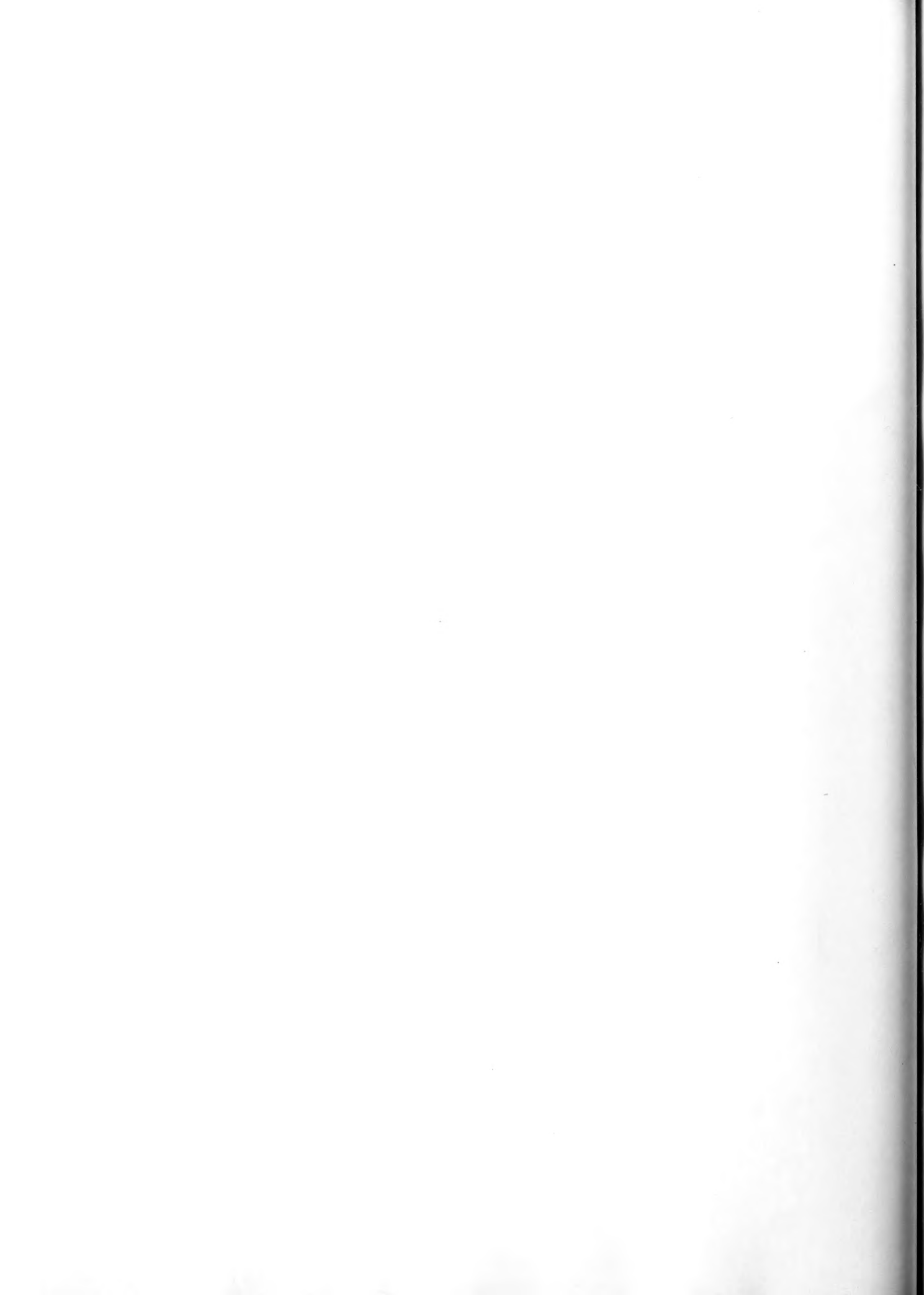


Figure 8



Vertical Loading Frame



Figure 9



to insure that accurate results would be obtained. Therefore, an extruded "T" beam was obtained and subjected to a load test on the loading frame. The method used to check our procedure consisted of loading the beam and comparing the actual and computed deflections. This set up is shown in Figure 10.

The beam, when in the loading frame, was supported on knife edges which were rounded on the underneath side so that no restraint was placed on them. The load consisted of lead shot placed in a bucket. It was applied to the beam by means of a knife edge, attached to a yoke (see Figure 11) which supported the bucket. The deflection was measured by a 1/10,000 of an inch direct reading dial, placed underneath the mid-point of the span. The dial holder is shown in Figure 11 and was made such that it would also serve as a dial holder for taking readings on the horizontal loading frame.

A comparison of the actual deflections, under load, with the deflections computed by conventional formulae show an average difference of 1.4%. This check was considered close enough to allow the use of this vertical loading frame for future model tests.



to insure that accurate results would be obtained. Therefore, an extruded "T" beam was obtained and subjected to a load test on the loading frame. The method used to check our procedure consisted of loading the beam and comparing the actual and computed deflections. This set up is shown in Figure 10.

The beam, when in the loading frame, was supported on knife edges which were rounded on the underneath side so that no restraint was placed on them. The load consisted of lead shot placed in a bucket. It was applied to the beam by means of a knife edge, attached to a yoke (see Figure 11) which supported the bucket. The deflection was measured by a  $1/10,000$  of an inch direct reading dial, placed underneath the mid-point of the span. The dial holder is shown in Figure 11 and was made such that it would also serve as a dial holder for taking readings on the horizontal loading frame.

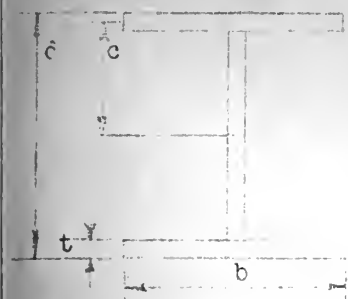
A comparison of the actual deflections, under load, with the deflections computed by conventional formulae show an average difference of 1.4%. This check was considered close enough to allow the use of this vertical loading frame for future model tests.

for future model tests.

close enough to allow the use of this vertical load. From  
show an average difference of 1.4%. It is shown that the error  
load, with the vertical load, is controlled by the vertical load.  
A comparison of the vertical load, with the vertical load,  
for taking results in the vertical load, with the vertical load.

It was also shown that the vertical load, with the vertical load,  
the mid-point of the beam. The vertical load, with the vertical load,  
1/10,000 of an inch direct stress. The vertical load, with the vertical load,  
which was a total of 100. The vertical load, with the vertical load,  
means for a better understanding of the vertical load, with the vertical load,  
lead to a better understanding of the vertical load, with the vertical load,  
that the vertical load, with the vertical load, with the vertical load,  
on the edge of the beam, with the vertical load, with the vertical load,  
for the vertical load, with the vertical load, with the vertical load.





All Dimensions are in inches.

Computations:

Moment of Inertia,

$$I_{\text{flange}} = \frac{2(bt^3 + btc^2)}{12}, \quad I_{\text{web}} = \frac{t(d-2t)^3}{12}$$

I Total equals I flange + I web.

Deflection,

$$D = \frac{PL^3}{48 EI}$$

D- Deflection in inches  
L- Span length in inches  
E- Modulus of Elasticity  
I- Moment of Inertia  
P- Load in pounds

Stress

$$f = \frac{Mc}{I}$$

f- Stress in psi  
M- Moment in inch-pounds  
I- Moment of Inertia  
c- As shown above

Figure 10



Loading Yoke and Deflection  
Dial Holder

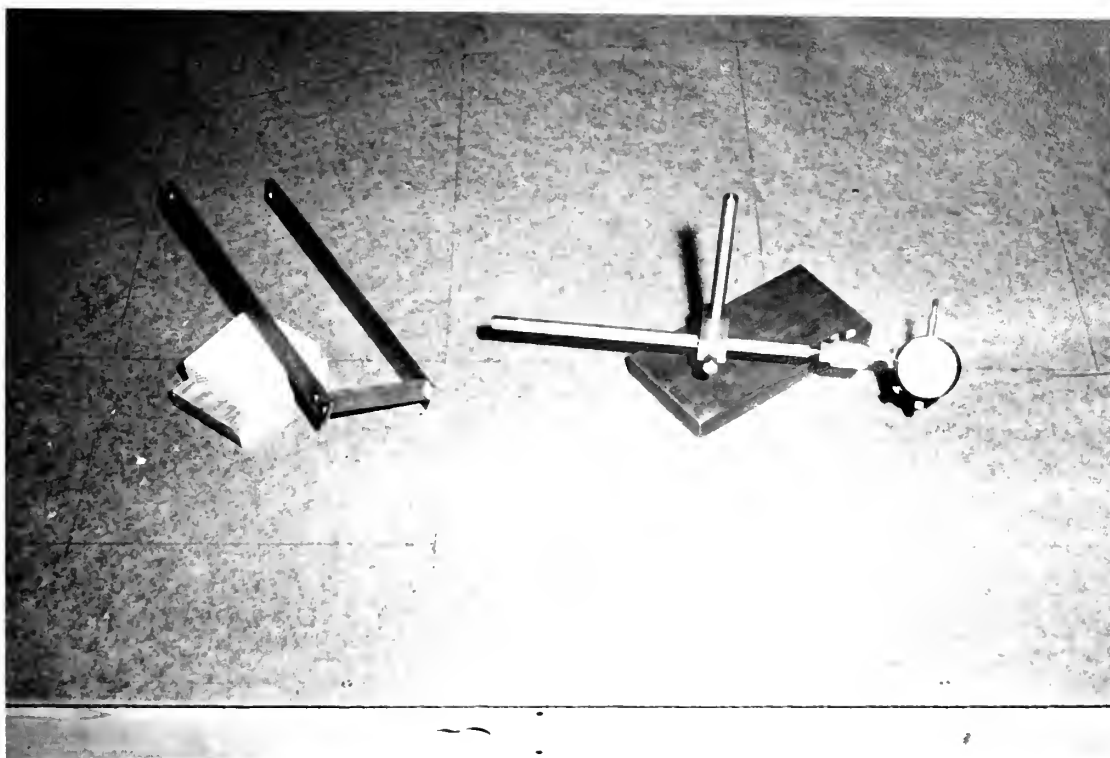


Figure 11



## Extruded "T" Beam

(Beam #1)

Dimensions of Beam  
(See Figure 10)

b = 1.24 inches  
t = .128 inches  
d = .87 inches  
L = 34 inches

Neutral Axis was computed to be .227 inches above the base.

Moment of inertia (I) = .0158 inches<sup>4</sup>

Deflection (D) = 5.19 P (10<sup>-3</sup>)

Stress (f) = 345 P

## Dial Reading

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Dif.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|---------------|---------------|
| 1.6         | .07551      | .08330        | .00801           | .00830            | 0             | 551.0         |
| 2.16        | .07566      | .08679        | .01128           | .01123            | .45           | 745.0         |
| 3.0         | .07555      | .09170        | .01604           | .01555            | 3.05          | 1035.0        |
| 4.0         | .07560      | .09708        | .02153           | .02073            | 3.72          | 1380.0        |
| 5.0         | .07560      | .10160        | .02600           | .02590            | .38           | 1720.0        |
| 7.0         | .07561      | .11201        | .03641           | .03630            | .32           | 2320.0        |
| 9.0         | .07561      | .12314        | .04753           | .04670            | 1.74          | 3100.0        |
| 11.0        | .07561      | .13320        | .05759           | .05700            | 1.02          | 3800.0        |
| 13.0        | .07561      | .14382        | .06821           | .06730            | 1.33          | 4480.0        |
| 15.0        | .07561      | .15489        | .07928           | .07775            | 1.93          | 5180.0        |

11-11-11

11-11-11

11-11-11

11-11-11

11-11-11

| Load | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
|------|----------|----------|----------|----------|----------|----------|
| 1.0  | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 2.0  | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 3.0  | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 4.0  | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 5.0  | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 6.0  | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 7.0  | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 8.0  | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 9.0  | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 10.0 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 11.0 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 12.0 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 13.0 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |
| 14.0 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 | 11-11-11 |

#### 4. Technique of Joining Flanges to Web

There were three methods used by the authors in fabricating models from aluminum. They were eutecrod welding, soldering, and furnace brazing. It is in this section that we will discuss the three methods and the results of the tests run on the models constructed by each method.

##### a. Eutecrod Welding

###### (1) Welding difficulties

The difficulty in welding with eutecrod is the high temperature required for fusion, which approaches the melting temperature of aluminum. In actual practice the two temperatures differ only by about 50 degrees and great care must be exercised not to bridge this differential. The parent material will warp and disintegrate very quickly when the melting point is approached. Another important point to consider is that, in the vicinity of the weld, the yield strength of the material has decreased considerably, resulting in the material no longer being homogeneous. These two facts are very important and must be considered in view of the final results desired.

###### (2) Flux

The flux used was supplied by the eutecrod company to be used in conjunction with their rod. It is a powder that is mixed with

two facts are very important and must be considered in view of the final results desired.

material no longer being homogeneous. These

has decreased considerably, resulting in the

of the weld, the yield strength of the material

tensile point is considered as lost, in the vicinity

melting point is approached. In fact, a

very and disintegrates very rapidly, even the

this differential. The various materials with

that there must be an expansion of the material

to be able to weld. This is the reason

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is that the material is not

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(1) The material is not

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is that the material is not

is that the material is not



water to form a paste, which is spread on the joint to be welded. Care must be exercised in applying the flux, insuring that only the surfaces at the joint are covered. This is true because, if too much is used, the flux allows the eutectoid to run as it melts, covering a weld area that is too large. This point is not essential in making a good weld but it appears to help.

### (3) Method of welding (See Figure 12)

The actual method used in welding is similar to that used in any torch welding, with a modification. The big change adopted was in the way the heat from the torch was applied to the joint. Rather than directing the flame almost perpendicular to the joint, we found it better to shoot the flame parallel to the joint, heating with the side of the flame. Using this method, it was found that there was better control of the heat, giving effective preheating with less chance of overheating. The rest of the welding procedure is the same, i.e., feeding in welding rod as the temperature gets high enough, and moving along fast enough to give an even fillet.

1955

Method of Welding



Figure 12



#### (4) Test Samples and Results

In order to be sure of the amount of load eutecrod welding would sustain, a series of test samples were made. They were of the form as shown in Figure 13 with the dimensions and results as shown below.

##### Shear test

a = 1 inch  
L = 2 inches  
b = 3/4 inch  
h = .091 inch

Under a load (P) of 1590 pounds, the parent material broke across the 3/4 inch dimension.

##### Tension test

a = 1 inch  
b = 3/4 inch  
h = .091 inch

Under a load (P) of 980 pounds, the weld broke.

The results of these tests were definite proof that any welds made with eutecrod were sufficiently strong to withstand more load than the parent material, and therefore, strong enough to carry the loads we would use.

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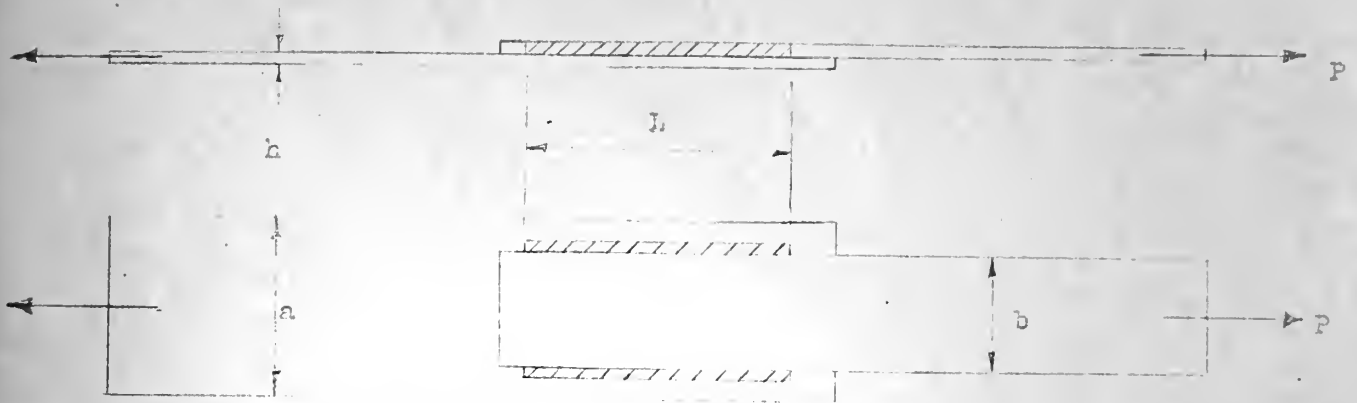
0954-6794(199701)100:1:1-0

0954-6794(199701)100:1:1-0

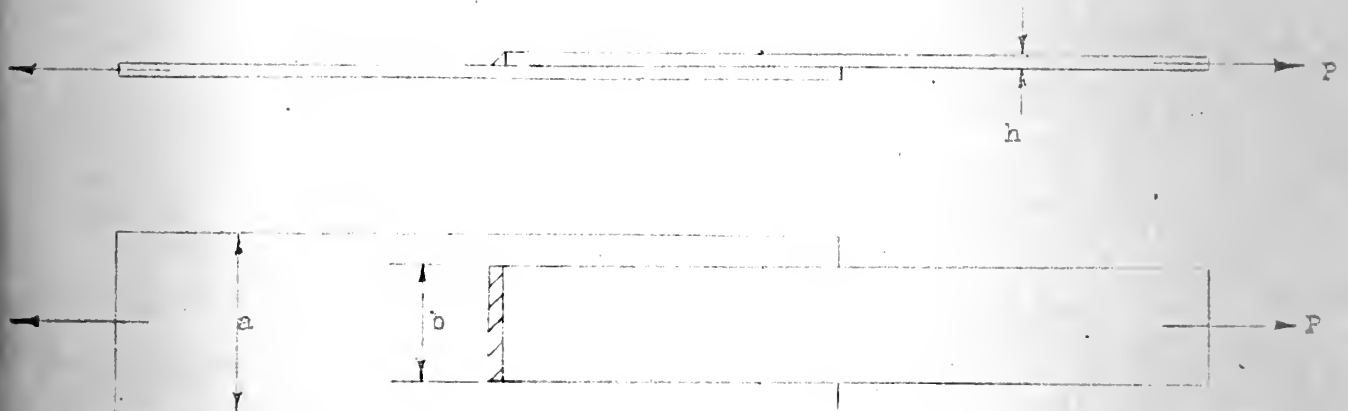
0954-6794(199701)100:1:1-0

0954-6794(199701)100:1:1-0

## Shear and Tension Test Specimen



Shear Test



Tension Test

Figure 13





## (5) Aluminum welded beam tests and results

## Beam #4

Beam #4 was constructed, using the eutecrod welding method, in jig #2. It was tested on the vertical loading frame with the results as given below.

Dimensions of Beam  
(See Figure 10)

b = 2.0 inches  
t = .093 inches  
c = 1.32 inches  
d = 2.73 inches  
L = 24 inches

Moment of Inertia (I) = .7750

Deflection (D) =  $.0371 \times 10^{-3} P$

Stress (f) = 10.2 P

## Dial Reading

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 10          | .2800       | .2831         | .0031            | .00037            | 87.0           | 102.0         |
| 26.6        | .2800       | .2845         | .0045            | .0010             | 78.0           | 272.0         |
| 35.0        | .2821       | .2859         | .0038            | .0013             | 66.0           | 357.0         |
| 50.6        | .2823       | .2875         | .0052            | .0019             | 63.0           | 516.0         |
| 60.0        | .2826       | .2882         | .0056            | .0022             | 61.0           | 612.0         |
| 75.6        | .2828       | .2901         | .0073            | .0028             | 62.0           | 771.0         |
| 85.0        | .2829       | .2916         | .0087            | .0032             | 63.0           | 867.0         |
| 100.6       | .2831       | .2928         | .0097            | .0037             | 62.0           | 1020.0        |
| 110.0       | .2832       | .2936         | .0104            | .0041             | 61.0           | 1121.0        |
| 125.6       | .2836       | .2946         | .0110            | .0047             | 57.0           | 1280.0        |
| 135.0       | .2839       | .2949         | .0110            | .0050             | 55.0           | 1379.0        |
| 150.6       | .2839       | .2976         | .0137            | .0056             | 59.0           | 1537.0        |

[illegible]

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 160.0       | .2844       | .2981         | .0137            | .0059             | 57.0           | 1631.0        |
| 185.0       | .2849       | .2997         | .0148            | .0068             | 54.0           | 1889.0        |
| 215.5       | .2850       | .3004         | .0154            | .0080             | 48.0           | 2195.0        |

Beam #4 was warped and distorted which accounts for the high percentage error. These high errors indicate that the whole method was entirely inadequate for a simple laboratory technique. The next attempt at eutecrod welding was Beam #7.



## Beam #7

Beam #7 was constructed using the autecrod welding method in jig #3. It was tested on the vertical loading frame with the results as follows:

Dimensions of Beam  
(See Figure 10)

b = 1.03 inches  
t = .063 inches  
c = .55 inches  
d = 1.16 inches  
L = 12 inches

Moment of Inertia (I) = .0455

Deflection (D) =  $.0791 \times 10^{-3}$  P

Stress (f) = 36.2 P

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 1.6         | .2337       | .2341         | .0004            | .000127           | 67.5           | 58.0          |
| 5           | .2337       | .2347         | .0010            | .000396           | 60.4           | 181.0         |
| 12.2        | .2337       | .2357         | .0020            | .000963           | 52.0           | 442.0         |
| 19.85       | .2335       | .2364         | .0029            | .00157            | 48.3           | 719.0         |
| 27.85       | .2338       | .2373         | .0035            | .00220            | 37.0           | 1005.0        |
| 35.7        | .2338       | .2379         | .0041            | .00283            | 31.0           | 1290.0        |
| 42.65       | .2340       | .2389         | .0049            | .00337            | 31.2           | 1540.0        |
| 51.1        | .2340       | .2400         | .0060            | .00403            | 32.8           | 1850.0        |
| 58.85       | .2341       | .2410         | .0069            | .00465            | 32.6           | 2130.0        |
| 66.35       | .2343       | .2416         | .0073            | .00525            | 28.7           | 2400.0        |
| 74.65       | .2341       | .2428         | .0087            | .00590            | 32.2           | 2750.0        |
| 110.00      | .2339       | .2470         | .0130            | .00870            | 33.1           | 3980.0        |
| 160.0       | .2340       | .2518         | .0175            | .01265            | 28.0           | 5800.0        |

1. The first part of the report is a general statement of the work done during the year.  
 2. The second part is a detailed account of the work done in each of the departments.  
 3. The third part is a statement of the financial results of the year.

4. The fourth part is a statement of the work done in each of the departments.  
 5. The fifth part is a statement of the financial results of the year.

6. The sixth part is a statement of the work done in each of the departments.  
 7. The seventh part is a statement of the financial results of the year.

8. The eighth part is a statement of the work done in each of the departments.  
 9. The ninth part is a statement of the financial results of the year.

| Loaded | Loaded | Loaded | Loaded | Loaded | Loaded | Loaded |
|--------|--------|--------|--------|--------|--------|--------|
| 1.0    | 1.0    | 1.0    | 1.0    | 1.0    | 1.0    | 1.0    |
| 2      | 2      | 2      | 2      | 2      | 2      | 2      |
| 18.8   | 18.8   | 18.8   | 18.8   | 18.8   | 18.8   | 18.8   |
| 19.8   | 19.8   | 19.8   | 19.8   | 19.8   | 19.8   | 19.8   |
| 27.8   | 27.8   | 27.8   | 27.8   | 27.8   | 27.8   | 27.8   |
| 38.7   | 38.7   | 38.7   | 38.7   | 38.7   | 38.7   | 38.7   |
| 48.6   | 48.6   | 48.6   | 48.6   | 48.6   | 48.6   | 48.6   |
| 51.1   | 51.1   | 51.1   | 51.1   | 51.1   | 51.1   | 51.1   |
| 58.8   | 58.8   | 58.8   | 58.8   | 58.8   | 58.8   | 58.8   |
| 68.3   | 68.3   | 68.3   | 68.3   | 68.3   | 68.3   | 68.3   |
| 74.8   | 74.8   | 74.8   | 74.8   | 74.8   | 74.8   | 74.8   |
| 110.0  | 110.0  | 110.0  | 110.0  | 110.0  | 110.0  | 110.0  |
| 180.0  | 180.0  | 180.0  | 180.0  | 180.0  | 180.0  | 180.0  |

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 185.0       | .2343       | .2540         | .0197            | .01462            | 25.8           | 6700.0        |
| 235.0       | .2350       | .2603         | .0253            | .01860            | 26.5           | 8500.0        |
| 259.8       | .2362       | .2644         | .0282            | .0205             | 27.4           | 9370.0        |
| 283.0       | .2362       | .2680         | .0318            | .0223             | 29.9           | 10,500.0      |

The readings taken on Beam #7 were consistently better than those on Beam #4, but the percentage error was still much too high to accept this method as a way for building models. It appears that, due to the localized heating, there is a definite zone of softening in the area of the weld which caused the beam to act irregularly.





## b. Aluminum Soldering

The method of using aluminum solder as a means of constructing our model beams was investigated at the same time as the eutecrod method. The two methods are very similar, and their similarities along with the differences will be presented in this section.

### (1) Characteristics of Solder

Alladin soldering is not as strong as eutecrod welding. However, once the limitations were discovered, it was possible to use it with considerable success. The solder melts at a much lower temperature than does welding rod. This most important characteristic makes it much easier to use since the melting point of the parent material is not approached. However, since there is no direct fusion of material, the strength of the joint is definitely decreased. The sample pull test results will indicate this much more clearly. The rod used was an alladin rod. This particular rod required no flux. Therefore, it was necessary to insure that all oxides were cleaned off the aluminum prior to soldering. In addition, a reducing flame was used to prevent the formation of any oxides while soldering.

1. The first step in the process of soldering is the preparation of the surfaces to be joined. This involves the removal of any oxide film that may be present on the surfaces. This is usually done by mechanical means, such as sandblasting or wire brushing. In some cases, chemical cleaning may be used. The surfaces must be clean and free of any contaminants before soldering can proceed.

2. The second step is the application of flux. Flux is a substance that prevents the formation of oxide film during the soldering process. It is usually applied as a paste or a liquid. The flux must be applied to the surfaces before the solder is applied.

3. The third step is the application of solder. The solder is usually applied in the form of a wire or a paste. The solder must be applied to the surfaces while the flux is still present. The solder will then flow into the joint and form a bond between the two surfaces.

4. The fourth step is the removal of the flux. The flux must be removed from the joint after the soldering process is complete. This is usually done by washing the joint with a solvent. The solvent must be able to dissolve the flux without damaging the solder or the surfaces.

5. The fifth step is the inspection of the joint. The joint must be inspected to ensure that it is properly formed and that there are no defects. This is usually done by visual inspection. The joint should be free of any defects, such as voids or cracks.

6. The sixth step is the testing of the joint. The joint must be tested to ensure that it is strong enough to withstand the stresses and strains that it will be subjected to in service. This is usually done by mechanical testing.

7. The seventh step is the documentation of the process. The process must be documented to ensure that it can be repeated in the future. This is usually done by writing a procedure manual.

The method of cleaning the aluminum was the same as that outlined under eutecrod welding. Of the two size rods available, the 1/16" rod was preferred to the 1/8" rod due to the size of the sections being joined. There was only one difficulty encountered, other than those mentioned under eutecrod welding. It was noticed that the solder already placed tended to ball up in some places along the joint when placing solder on the opposite side. This occurred only in a few locations, however, and was patched up easily by reheating and soldering.

## (2) Method of Soldering

The technique of heating the joint in preparation for soldering was the same as outlined under the method of eutecrod welding. Since the solder requires a lower temperature than eutecrod, the size of the flame used was considerably smaller. The pressure settings on the cylinder regulators were 5 lbs. and 2 lbs. for oxygen and acetylene respectively. The main difference in soldering is when the filler rod is added. As the torch is held in position for heating the joint, it is best to hold the filler rod in the outer fringe of the flame to keep it

The first step in the process of welding is to clean the surfaces to be joined. This is done by removing any oil, dirt, or other contaminants that might interfere with the welding process. The next step is to heat the metal to a temperature at which it becomes plastic. This is done by using a torch or other heat source. Once the metal is heated, the filler metal is added to the joint. The filler metal is a metal that has a lower melting point than the base metal. It is added to the joint in the form of a wire or rod. The heat from the torch causes the filler metal to melt and flow into the joint. The final step is to allow the joint to cool and solidify. This process is called brazing.

#### (b) Method of brazing

The technique of brazing is similar to that of welding, but the temperature used is lower. In brazing, the base metal is heated to a temperature at which it becomes plastic, and the filler metal is added to the joint. The filler metal is a metal that has a lower melting point than the base metal. It is added to the joint in the form of a wire or rod. The heat from the torch causes the filler metal to melt and flow into the joint. The final step is to allow the joint to cool and solidify. This process is called brazing. The main difference between brazing and welding is the temperature used. In welding, the temperature is high enough to cause the base metal to melt. In brazing, the temperature is lower, and the base metal does not melt. The filler metal is the only metal that melts. The pressure settings on the cylinder regulators were 5 lbs. and 3 lbs. for oxygen and acetylene respectively. The main difference in brazing is when the filler rod is added. As the torch is held in position for heating the joint, it is best to hold the filler rod in the outer fringe of the flame to keep it

in a soft condition. Then, when the reflected flame turns orange, quickly remove the torch and wipe the filler rod along the joint. It will be possible only to run the joint for about 1 to 2 inches, as the metal cools quickly. However, in our method of using the torch to preheat as well as weld, it will be necessary just to heat the joint a second or two until it will be hot enough again to make another run. This procedure is continued until the whole length of weld is completed.

### (3) Test Samples and Results (See Figure 13)

A series of tests on samples, similar to those run using eutecrod, were run using solder. Since there is quite a range of temperatures at which the solder will flow and still not affect the parent material, we ran two sample tests. The first was on a model soldered at a very high temperature such that there was almost fusion. The second was run at the lowest possible temperature such that there was no fusion. The results of these two tests were considered as limits of the possible strength a soldered joint would take. In all future tests, we kept our horizontal shear definitely below that indicated by the lowest test.

that indicated by the lowest test. kept our horizontal shear definitely below joint would take. In all future tests, we as limits of the possible strength a soldered results of these two tests were considered form such that there was no fusion. The second was run at the lowest possible temper- and that there was almost fusion. The a good soldered is a very high temperature we had two soldered joints. The first of and still soldered. In all respects, for comparison as we had the first. The solder. It is seen in figure 1 range of those tested. However, there is a slight

Test #1 (Fusion of material and solder noted)

|            |          |        |
|------------|----------|--------|
| Shear test | a = 1    | inch   |
|            | L = 2    | inches |
|            | b = 3/4  | inch   |
|            | h = .091 | inch   |

Under a load (P) of 1176 pounds the solder failed in shear

|              |          |      |
|--------------|----------|------|
| Tension test | a = 1    | inch |
|              | b = 3/4  | inch |
|              | h = .091 | inch |

Under a load (P) of 85 pounds, the solder failed.

Test #2 (Low temperature)

|            |          |        |
|------------|----------|--------|
| Shear test | a = 1    | inch   |
|            | L = 2    | inches |
|            | b = 3/4  | inch   |
|            | h = .091 | inch   |

The load built up to 50.5 pounds, then the solder yielded suddenly within the joint, although no cracks were visible. It was impossible to make the specimen take any more load. The horizontal shear was 12.6 lbs./inch.

The authors concluded from these tests that if the alladin solder method were to be used it would be necessary to keep the loads down such that the horizontal shear would be less than 12 lbs./inch, except where we were interested in the beam behavior at higher loads.

|   |   |   |   |   |   |   |   |   |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |     |
|---|---|---|---|---|---|---|---|---|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|-----|
| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 | 51 | 52 | 53 | 54 | 55 | 56 | 57 | 58 | 59 | 60 | 61 | 62 | 63 | 64 | 65 | 66 | 67 | 68 | 69 | 70 | 71 | 72 | 73 | 74 | 75 | 76 | 77 | 78 | 79 | 80 | 81 | 82 | 83 | 84 | 85 | 86 | 87 | 88 | 89 | 90 | 91 | 92 | 93 | 94 | 95 | 96 | 97 | 98 | 99 | 100 |
|---|---|---|---|---|---|---|---|---|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|-----|

1. The first part of the document is a list of names and titles, including "The Hon. Mr. Justice" and "The Hon. Mr. Justice".

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The above information was obtained from the files of the Department of the Interior, Bureau of Land Management, and is being furnished to you for your information.

The authors concluded from these tests that if the alabaster soldier nesting were to be used it would be necessary to keep the loads down such that the horizontal shear would be less than 12 lbs./in<sup>2</sup>, except where we were interested in the beam behavior at higher loads.

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## (4) Aluminum soldered beam tests and results

## Beam #2

Beam #2 was constructed using the alladin solder method in Jig #1. It was tested on the vertical loaded frame.

Dimension of Beam  
(See Figure 10)

b = 1.03 inches  
t = .064 inches  
c = .55 inches  
d = 1.16 inches  
L = 14 inches

Moment of inertia (I) = .0455

Deflection (D) =  $.1256 \times 10^{-3} P$

Stress (f) = 42.3 P

## Dial Reading

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 15.1        | .600        | .5975         | .0025            | .0019             | 24.0           | 640.0         |
| 45.75       | .600        | .5925         | .0075            | .0057             | 24.0           | 1940.0        |
| 84.0        | .600        | .5860         | .0140            | .0105             | 25.0           | 3580.0        |
| 100.60      | .600        | .5835         | .0165            | .0126             | 23.6           | 4260.0        |
| 116.35      | .600        | .5810         | .0190            | .0146             | 23.1           | 4930.0        |
| 131.6       | .600        | .5785         | .0215            | .0165             | 23.1           | 5570.0        |
| 156.6       | .600        | .5742         | .0258            | .0196             | 24.0           | 6640.0        |
| 166.35      | .600        | .5725         | .0275            | .0209             | 24.0           | 7060.0        |
| 174.65      | .600        | .5709         | .0291            | .0219             | 24.8           | 7400.0        |
| 188.75      | .600        | .5684         | .0316            | .0237             | 25.0           | 7999.0        |
| 202.75      | .600        | .5658         | .0342            | .0255             | 25.5           | 8560.0        |
| 210.85      | .600        | .5632         | .0368            | .0265             | 27.9           | 8960.0        |

The error in Beam #2 was believed to have resulted from the fact that the beam was warped and untrue. We, therefore, constructed beam #3.

constructed beam #2.  
 the fact that the beam was warped and untrue. We, therefore,  
 The error in beam #2 was believed to have resulted from

| Load   | Time | Distance | Time | Distance | Time | Distance |
|--------|------|----------|------|----------|------|----------|
| 15.1   | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 45.75  | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 84.0   | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 100.00 | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 116.38 | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 131.0  | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 150.6  | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 160.35 | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 174.65 | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 188.75 | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 202.75 | .60  | .6075    | .60  | .6075    | .60  | .6075    |
| 210.85 | .60  | .6075    | .60  | .6075    | .60  | .6075    |

## Beam #3

Beam #3 was constructed using the alladin soldering method in Jig #1. It was tested on the vertical loading frame.

Dimensions of Beam  
(See Figure 10)

b = 1.03 inches  
t = .064 inch  
c = .55 inch  
d = 1.15 inches  
L = 23 inches

Moment of Inertia (I) = .0452

Deflection (D) =  $.558 \times 10^{-3} P$

Stress (f) = 73.2 P

## Dial Reading

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 1.6         | .1559       | .1568         | .0009            | .0008             | 11.0           | 117.0         |
| 5.0         | .1554       | .1601         | .0047            | .0028             | 40.3           | 366.0         |
| 12.2        | .1554       | .1649         | .0095            | .0068             | 28.4           | 893.0         |
| 19.8        | .1553       | .1692         | .0139            | .0111             | 20.1           | 1455.0        |
| 27.8        | .1555       | .1741         | .0186            | .0159             | 15.0           | 2040.0        |
| 35.70       | .1556       | .1789         | .0233            | .0199             | 14.6           | 2610.0        |
| 43.50       | .1556       | .1839         | .0283            | .0243             | 14.2           | 3190.0        |
| 51.25       | .1563       | .1893         | .0330            | .0286             | 13.3           | 3750.0        |
| 58.75       | .1575       | .1978         | .0403            | .0328             | 18.5           | 4300.0        |
| 83.75       | .1605       | .2440         | .0835            | .0418             | 49.3           | 6130.0        |

Beam #3 was made considerably longer than Beam #2. This, along with the fact that a zero reading was taken after each load, had a noticeable effect on the results. While these results were better, it was decided to try a new method. We next built beam #5.

Method of determining the deflection of a beam under a load.

Dimensions of beam (see figure 1)  
 Length = 1.00 m  
 Width = 0.05 m  
 Height = 0.05 m

Force of weight  $(F) = 1.00 \text{ N}$   
 Deflection  $(\delta) = .50 \times 10^{-2} \text{ m}$   
 Stress  $(\sigma) = 15.0 \text{ N/m}^2$

Table 1

| Load | Deflected | Force | Deflected | Force | Deflected |
|------|-----------|-------|-----------|-------|-----------|
| 1.0  | 1.550     | 1.000 | 1.550     | 1.000 | 1.550     |
| 2.0  | 1.554     | 1.001 | 1.554     | 1.001 | 1.554     |
| 3.0  | 1.558     | 1.002 | 1.558     | 1.002 | 1.558     |
| 4.0  | 1.562     | 1.003 | 1.562     | 1.003 | 1.562     |
| 5.0  | 1.566     | 1.004 | 1.566     | 1.004 | 1.566     |
| 6.0  | 1.570     | 1.005 | 1.570     | 1.005 | 1.570     |
| 7.0  | 1.574     | 1.006 | 1.574     | 1.006 | 1.574     |
| 8.0  | 1.578     | 1.007 | 1.578     | 1.007 | 1.578     |
| 9.0  | 1.582     | 1.008 | 1.582     | 1.008 | 1.582     |
| 10.0 | 1.586     | 1.009 | 1.586     | 1.009 | 1.586     |
| 11.0 | 1.590     | 1.010 | 1.590     | 1.010 | 1.590     |
| 12.0 | 1.594     | 1.011 | 1.594     | 1.011 | 1.594     |
| 13.0 | 1.598     | 1.012 | 1.598     | 1.012 | 1.598     |
| 14.0 | 1.602     | 1.013 | 1.602     | 1.013 | 1.602     |
| 15.0 | 1.606     | 1.014 | 1.606     | 1.014 | 1.606     |
| 16.0 | 1.610     | 1.015 | 1.610     | 1.015 | 1.610     |
| 17.0 | 1.614     | 1.016 | 1.614     | 1.016 | 1.614     |
| 18.0 | 1.618     | 1.017 | 1.618     | 1.017 | 1.618     |
| 19.0 | 1.622     | 1.018 | 1.622     | 1.018 | 1.622     |
| 20.0 | 1.626     | 1.019 | 1.626     | 1.019 | 1.626     |

Beam #2 was made considerably longer than Beam #1. This, along with the fact that a zero reading was taken after each load, had a noticeable effect on the results. While these results were better, it was decided to try a new method. We next built beam #3.

## Beam #5

Beam #5 was constructed using the alladin soldering method in Jig #2. It was tested on the vertical loading frame.

Dimensions of Beam  
(See Figure 10)

b = 2.0 inches  
t = .091 inches  
c = 1.28 inches  
d = 2.75 inches  
L = 33 inches

Moment of Inertia (I) = .7715

Deflection (D) =  $.097 \times 10^{-3} P$

Stress (f) = 13.7 P

## Dial Reading

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 1.6         | .09037      | .09056        | .00019           | .00016            | 15.8           | 21.9          |
| 5.0         | .09041      | .09110        | .00069           | .00048            | 30.4           | 68.5          |
| 13.0        | .09041      | .09193        | .00143           | .00126            | 11.9           | 178.0         |
| 20.5        | .09041      | .09337        | .00296           | .00199            | 32.8           | 280.0         |
| 27.7        | .09062      | .09395        | .00333           | .00269            | 19.2           | 379.0         |
| 34.65       | .09041      | .09505        | .00464           | .00336            | 27.6           | 473.0         |
| 42.50       | .09060      | .09619        | .00559           | .00412            | 26.3           | 583.0         |
| 50.25       | .09060      | .09752        | .00692           | .00487            | 29.6           | 689.0         |
| 58.05       | .09081      | .09870        | .00789           | .00563            | 33.2           | 795.0         |
| 65.20       | .09090      | .09959        | .00869           | .00632            | 27.3           | 895.0         |
| 73.60       | .09102      | .10071        | .00969           | .00713            | 26.7           | 1010.0        |
| 81.90       | .09113      | .10169        | .01056           | .00794            | 24.8           | 1120.0        |

Beam #5 was made longer than Beam #3, and of larger section. We made these changes to discover if perhaps length or size of section had a major effect on the results. The irregular results proved nothing other than it didn't appear to be acting as a beam. We next built beam #6.

| Load  | Beam  | Beam  | Beam  | Beam  | Beam  | Beam  |
|-------|-------|-------|-------|-------|-------|-------|
| 1.6   | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 2.0   | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 12.0  | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 20.5  | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 27.7  | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 34.65 | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 42.50 | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 50.25 | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 58.05 | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 65.20 | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 73.60 | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |
| 81.90 | .0007 | .0007 | .0007 | .0007 | .0007 | .0007 |

We made these changes to discover if perhaps length of size of section had a major effect on the results. The irregular results proved nothing other than it didn't appear to be acting as a beam. We next built beam #6.

## Beam #6

Beam #6 was constructed using the alladin soldering method in Jig #2. It was tested on the vertical loading frame.

Dimensions of Beam  
(See Figure 10)

b = 2.01 inches  
t = .091 inches  
c = .96 inches  
d = 2.10 inches  
L = 26 inches

Moment of Inertia (I) = .424

Deflection (D) =  $.0864 \times 10^{-3} P$

Stress (f) = 15.7 P

## Dial Reading

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 1.6         | .05533      | .05543        | .00010           | .00014            | 40.0           | 25.1          |
| 10.0        | .05545      | .05626        | .00081           | .00085            | 4.9            | 157.0         |
| 18.4        | .05546      | .05645        | .00199           | .00157            | 21.1           | 289.0         |
| 25.9        | .05550      | .05834        | .00284           | .00221            | 22.2           | 407.0         |
| 33.65       | .05580      | .05968        | .00388           | .00287            | 26.0           | 528.0         |
| 41.65       | .05605      | .06040        | .00435           | .00355            | 18.4           | 653.0         |
| 48.80       | .05608      | .06110        | .00502           | .00417            | 16.9           | 765.0         |
| 64.55       | .05610      | .06384        | .00774           | .00557            | 28.0           | 1010.0        |
| 73.80       | .05665      | .06533        | .00868           | .00636            | 26.8           | 1158.0        |

Beam #6 didn't eliminate the errors although the percentage error was less than that occurring in beam #5.

Beam #6 didn't eliminate the errors although the percentages

error was less than that occurring in beam #5.

| Load  | Zero   | Load   | Zero   | Load   | Zero   | Load   | Zero   |
|-------|--------|--------|--------|--------|--------|--------|--------|
| 1.6   | .05835 | .05845 | .00010 | .00014 | .00014 | .00014 | .00014 |
| 10.0  | .05844 | .05836 | .00001 | .00001 | .00001 | .00001 | .00001 |
| 18.4  | .05846 | .05843 | .00003 | .00003 | .00003 | .00003 | .00003 |
| 25.9  | .05850 | .05834 | .00004 | .00004 | .00004 | .00004 | .00004 |
| 33.65 | .05850 | .05838 | .00008 | .00008 | .00008 | .00008 | .00008 |
| 41.65 | .05805 | .06040 | .00435 | .00435 | .00435 | .00435 | .00435 |
| 48.80 | .05808 | .06110 | .00302 | .00302 | .00302 | .00302 | .00302 |
| 64.55 | .05810 | .06384 | .00574 | .00574 | .00574 | .00574 | .00574 |
| 73.80 | .05865 | .06533 | .00668 | .00668 | .00668 | .00668 | .00668 |

1.6  
 10.0  
 18.4  
 25.9  
 33.65  
 41.65  
 48.80  
 64.55  
 73.80

Percent of zero = .00014  
 Deflection ( ) = .00001  
 Stress ( ) = 15.7



## Beam #8

Beam #8 was constructed, using the alladin solder method in Jig #3. It was tested on the vertical loading frame.

Dimensions of Beam  
(See Figure 10)

b = 1.03 inches  
c = .55 inches  
d = 1.16 inches  
t = .063 inches  
L = 24 inches

Moment of inertia (I) = .0454

Deflection (D) =  $.634 \times 10^{-3} P$

Stress (f) = 76.7 P

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 1.6         | .2825       | .2835         | .001             | .00101            | 1.0            | 122.5         |
| 5.0         | .2825       | .2857         | .0032            | .00317            | .94            | 383.0         |
| 12.2        | .2825       | .2912         | .0087            | .00773            | 11.5           | 931.0         |
| 19.85       | .2825       | .2964         | .0139            | .0126             | 9.35           | 1520.0        |
| 27.85       | .2830       | .3023         | .0198            | .0177             | 10.60          | 2130.0        |
| 35.70       | .2850       | .3077         | .0247            | .0227             | 8.10           | 2740.0        |
| 43.50       | .2833       | .3130         | .0300            | .0276             | 8.00           | 3330.0        |
| 51.25       | .2835       | .3183         | .0350            | .0325             | 7.13           | 3930.0        |
| 58.75       | .2835       | .3234         | .0399            | .0373             | 6.52           | 4500.0        |
| 65.90       | .2835       | .3286         | .0451            | .0408             | 9.53           | 5050.0        |
| 74.20       | .2841       | .3341         | .0503            | .0470             | 6.50           | 5680.0        |
| 82.35       | .2840       | .3399         | .0558            | .0521             | 6.71           | 6320.0        |
| 90.65       | .2847       | .3459         | .0619            | .0575             | 7.1            | 6950.0        |

The percentage error as indicated in beam #8 averages less than 10 percent. This indication that our methods and



techniques were improving convinced us that we should continue with our tests with only slight changes in our methods. It should be noticed that the beam is 24 inches long and of such a section that a large deflection is obtained. It is felt that a large deflection is necessary so that any errors that do occur are not a significant part of the deflection. It should also be noted that the horizontal shear on this beam reached 36.2 lbs./inches which is considerably above the absolutely safe value as determined by test. Therefore, in any future tests, a horizontal shear maximum of 10 lbs./inch can be assumed to be absolutely safe. With the above considerations in mind we constructed beam #9.

1. The first of these is the fact that the majority of the population of the United States is of European descent. This is a fact which is often overlooked in discussions of the Negro problem. It is a fact which is of great importance in determining the social and economic conditions of the Negro population. The majority of the population of the United States is of European descent, and this fact is of great importance in determining the social and economic conditions of the Negro population. The majority of the population of the United States is of European descent, and this fact is of great importance in determining the social and economic conditions of the Negro population.

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## Beam #9

Beam #9 was a "T" beam constructed using the alladin solder method in Jig #3 modified. It was tested on the vertical loading frame.

Dimensions of Beam  
(See Figure 10)

b = 1.453 inches  
c = .8244 inches  
d = 1.0938 inches  
t = .091 inches  
L = 54 inches

Moment of inertia (I) = .02375

Deflection (D) =  $1.38 \times 10^{-2} P$

Stress (f) = 468 P

Horizontal Shear (H) =  $\frac{VQ}{I} = .623 P$

The neutral axis was computed to be .268 inches above the base.

## Dial Reading

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 1.6         | .16340      | .18820        | .0248            | .0221             | 10.9           | 747.0         |
| 2.0         | .16340      | .19210        | .0287            | .0276             | 3.8            | 935.0         |
| 3.0         | .16340      | .20710        | .0437            | .0414             | 5.24           | 1405.0        |
| 4.0         | .16340      | .22010        | .0567            | .0552             | 2.45           | 1870.0        |
| 5.0         | .16340      | .23420        | .0708            | .0690             | 2.54           | 2340.0        |

The error in beam #9 was not as great as that in beam #8. The horizontal shear at 5 lbs. was 3.115 lbs./inches. We stopped loading at 5 lbs. as we wanted to put another flange on the "T" beam to see the effect. Therefore, we soldered a flange on beam #9 to get beam #10.

The error in beam #9 was not as great as that in beam #8.

The horizontal shear at 8 lbs. was 3.115 lbs./inches.

stopped loading at 8 lbs. as we wanted to put another flange on the "T" beam to see the effect. Therefore, we soldered a flange on beam #9 to get beam #10.

The central axis was supported at 1.388 inches above the base.

Horizontal shear (1) =  $\frac{1}{2} \times 1.388 = .694$

Deflection (1) =  $1.388 \times 10^{-3}$

Deflection (2) =  $1.388 \times 10^{-3}$

Dial Readings

| Load | Gage   | Loaded | Def. Def. | Comp. Def. | Dist. Def. |
|------|--------|--------|-----------|------------|------------|
| 1.6  | .16340 | .16380 | .0348     | .0341      | 10.4       |
| 2.0  | .16340 | .16310 | .0337     | .1376      | 8.8        |
| 3.0  | .16340 | .16310 | .0337     | .0414      | 5.34       |
| 4.0  | .16340 | .16310 | .0337     | .0333      | 3.48       |
| 5.0  | .16340 | .16320 | .0308     | .0300      | 3.34       |

The error in beam #9 was not as great as that in beam #8.

The horizontal shear at 8 lbs. was 3.115 lbs./inches.

stopped loading at 8 lbs. as we wanted to put another flange on the "T" beam to see the effect. Therefore, we soldered a flange on beam #9 to get beam #10.

## Beam #10

Beam #10 is beam #9 with another flange soldered on.

Dimensions of Beam  
(See Figure 10)

b = 1.453 inches  
c = .548 inches  
d = 1.188 inches  
t = .091 inches  
L = 54 inches

Moment of inertia (I) = .08873

Deflection (D) =  $3.7 \times 10^{-3}$  P

Stress (f) = 90.3 P

Horizontal Shear (H) = .408 P

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 1.6         | .1320       | .1382         | .0062            | .0059             | 4.8            | 144.5         |
| 2.0         | .1319       | .1397         | .0078            | .0074             | 5.1            | 180.5         |
| 3.0         | .1320       | .14285        | .01085           | .0111             | 2.3            | 271.0         |
| 4.0         | .1320       | .1469         | .0149            | .0148             | 0.6            | 361.0         |
| 5.0         | .1320       | .1500         | .0180            | .0185             | 2.8            | 451.0         |
| 6.0         | .1320       | .1541         | .0221            | .0222             | 0.4            | 542.0         |
| 7.0         | .1320       | .1580         | .0260            | .0259             | 0.4            | 631.0         |
| 8.0         | .1320       | .1620         | .0300            | .0296             | 1.3            | 722.0         |
| 9.0         | .1320       | .1658         | .0338            | .0333             | 1.5            | 811.0         |
| 10.0        | .1320       | .1697         | .0377            | .0370             | 1.9            | 903.0         |
| 18.0        | .1321       | .1998         | .0678            | .0666             | 1.8            | 1628.0        |

The small amount of difference between computed and actual deflections as evidenced by the percentage error was considered excellent. The horizontal shear obtained was 7.35 lbs./inches at 18 lbs. In view of the results, it was decided to construct a beam of larger cross section, to see if there would be any effect on the accuracy.





## Beam #11

Beam #11 was constructed using the alladin solder method in Jig #3 modified. It was tested on the vertical loading frame.

Dimensions of Beam  
(See Figure 10)

b = 1.234 inches  
c = 1.193 inches  
d = 2.477 inches  
t = .091 inches  
L = 54 inches

Moment of Inertia (I) = .4128

Deflection (D) =  $.798 \times 10^{-3} P$

Stress (f) = 40.5 P

Horizontal Shear (H) = .1623 P

| Dial Reading |             |               |                  |                   |                |               |
|--------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| <u>Load</u>  | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
| 5            | .0964       | .1001         | .0037            | .0040             | 8.1            | 202.0         |
| 10           | .0965       | .1041         | .0076            | .0080             | 5.3            | 404.0         |
| 15           | .0965       | .1083         | .0118            | .0120             | 1.7            | 616.0         |
| 20           | .0965       | .1122         | .0157            | .0160             | 1.9            | 807.0         |
| 25           | .0965       | .1164         | .0199            | .0199             | 0.0            | 1015.0        |
| 30           | .0967       | .1208         | .0243            | .0239             | 1.6            | 1215.0        |
| 35           | .0967       | .1250         | .0283            | .0279             | 1.4            | 1417.0        |
| 40           | .0967       | .1290         | .0323            | .0319             | 1.2            | 1620.0        |
| 45           | .0968       | .1333         | .0366            | .0359             | 1.9            | 1823.0        |
| 50           | .0969       | .1380         | .0412            | .0399             | 3.2            | 2020.0        |
| 55           | .0969       | .1419         | .0450            | .0439             | 2.4            | 2230.0        |

The results of Beam #11 proved that our methods and techniques of constructing models were satisfactory. This beam was made with the idea in mind of using it for checking stresses and calibrating the horizontal loading frame. (See Figure 14.) These tests are explained in section IV.

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| Load | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
|------|------|------|------|------|------|------|
| 5    | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 10   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 15   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 20   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 25   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 30   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 35   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 40   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 45   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 50   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |
| 55   | 1000 | 1000 | 1000 | 1000 | 1000 | 1000 |

The results of test 111 show that the test was successful.

Figures of construction, models were satisfactory. This was made with the idea in mind of using it for checking against the calibrating the horizontal loading frame. (See figure 14.)

These tests are explained in section IV.

Beam #11 on Horizontal  
Loading Frame

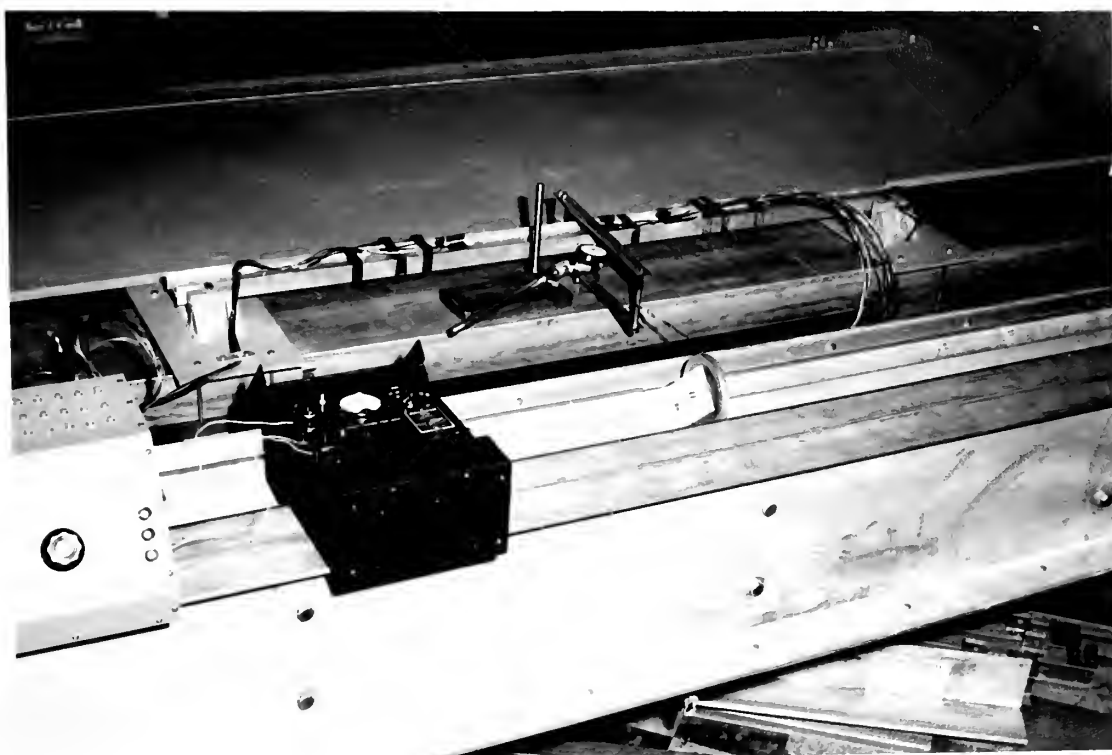


Figure 14



### c. Furnace Brazing

The third method of constructing beams by furnace brazing was not successful. The furnace used was the same one that was mentioned in the "E" check discussion.

The beam constructed was 24 inches long, 1-1/2 inches deep, .091 inches thick and the flanges were 1-1/4 inches wide. The beam was held together in the manner discussed for Jig #3 modified, with clamps holding the flanges together. The joining material used was eutecrod. In order to get a thin foil, the eutecrod was rolled to a thickness of about .008 inches. It was inserted in the joint and held in place by the clamping action of the "C" clamps. Flux was placed along all the surfaces that were to be joined. The furnace temperature used was 1125° F. This was the temperature, found from tests, that was necessary for the materials to fuze. The jugged beam was placed in the furnace and allowed to remain for 15 minutes. At the end of the required time, the furnace was shut off and allowed to cool before inspecting the beam.

The whole beam was completely distorted and warped. (See Figure 15.) The flanges were welded to the web for only about 2 inches at one end. It was impossible to test the beam because of its condition.

The purpose of this study is to investigate the effect of temperature on the mechanical properties of a material. The material in question is a type of metal alloy, and the properties of interest are its tensile strength and elongation. The study was conducted using a series of tensile tests at different temperatures, ranging from room temperature to a high temperature. The results of the tests are presented in the following sections. In the first section, the tensile strength of the material is discussed. It is shown that the tensile strength decreases as the temperature increases. This is due to the fact that the material becomes more ductile at higher temperatures. In the second section, the elongation of the material is discussed. It is shown that the elongation increases as the temperature increases. This is also due to the increased ductility of the material at higher temperatures. The third section discusses the effect of the rate of loading on the mechanical properties. It is shown that the tensile strength and elongation are both affected by the rate of loading. Finally, the fourth section discusses the effect of the size of the specimen on the mechanical properties. It is shown that the tensile strength and elongation are both affected by the size of the specimen. The results of the study are summarized in the following table:

| Temperature (°C) | Tensile Strength (MPa) | Elongation (%) |
|------------------|------------------------|----------------|
| 20               | 100                    | 10             |
| 100              | 90                     | 15             |
| 200              | 80                     | 20             |
| 300              | 70                     | 25             |
| 400              | 60                     | 30             |

Furnace Brazed Aluminum Beam



Figure 15





This method is impractical for use with aluminum. The eutectrod will not flow by itself until a temperature of about 1125° F. is reached. This temperature is above the melting point of the alloy used, and the beam will not even support its own weight. Thus, with the jiggling system used, the weight of the clamps alone caused the whole beam to be pulled out of shape. Therefore, the authors felt it a waste of time to attempt any further tests.

#### B. Steel.

For the fabrication of steel models, we selected hot rolled strip steel, 1-1/2 inches wide and 0.056 inches thick. This particular size material was selected from the available stock at a local steel yard because it would require the least cutting in the fabrication of a model. Hot rolled strip was chosen in preference to cold rolled strip because of its being relatively free of residual stresses.

##### 1. Preparation of Material

A hacksaw was used to cut the strip steel into the desired lengths. The rounded edges of the pieces were ground flat on a mechanical disc sander. Next, the scale on the edges and sides, where the pieces were to be joined, was removed by using emery cloth which gave a bright surface. Care should be exercised in grinding the edges to insure that a smooth, flat surface is obtained. Irregularities will cause a poor joint.



## 2. Jigs

Jigs used for making steel models were the same as those used for making aluminum models, as given in section III-A-2 above; consequently, they need not be discussed again in this section.

## 3. Techniques of Joining Flanges to Webs

### a. Silver soldering with an oxyacetylene torch

In joining the pieces of steel together to form a model, we wanted a strong joint, which could be obtained without heating the steel into its critical range. Heating to a low temperature was desirable also to avoid large expansions and accompanying distortions. Silver soldering seemed to possess all of the above desirable characteristics. The "Easy Flow" solder we used flowed freely at 1175° F., which is well below steel's critical temperature, and it possessed a tensile strength of approximately 65,000 psi.

#### (1) Joint thickness

In the "Welding Handbook" of the American Welding Society, a graph is shown expressing the strength of a soldered butt joint, using silver solder to join stainless steel, as a function of the joint thickness. With a joint thickness of 0.003 inches, the joint strength was 117,000 psi, while with a

The following table shows the results of the tests made on the joints of the American Welding Society, a graph is shown expressing the strength of a soldered butt joint, using silver solder to join stainless steel, as a function of the joint thickness. With a joint thickness of 0.003 inches, the joint strength was 117,000 psi, while with a

0.003 inch joint thickness, the joint strength was 117,000 psi.

The following table shows the results of the tests made on the joints of the American Welding Society, a graph is shown expressing the strength of a soldered butt joint, using silver solder to join stainless steel, as a function of the joint thickness. With a joint thickness of 0.003 inches, the joint strength was 117,000 psi, while with a

(1) Joint thickness

In the "Welding Handbook" of the American Welding Society, a graph is shown expressing the strength of a soldered butt joint, using silver solder to join stainless steel, as a function of the joint thickness. With a joint thickness of 0.003 inches, the joint strength was 117,000 psi, while with a

thickness of 0.024 inches, the strength was 47,000 psi. This shows the desirability of having a close fitting joint between the pieces being joined.

## (2) Heating and fluxing

Before the joint was heated, a coating of flux\* was painted on the surfaces to be joined, its purpose being to prevent oxidation of the solder and steel surfaces being joined, to dissolve any oxides that might form during heating, and to assist the flowing of the alloy. The flux also serves as a temperature indicator, in that the joint should be heated until the flux remains fluid if the torch flame is removed for an instant.

The models we made consisted of tee and wide-flange sections. In joining the web to the flange, the torch was held in a position so that the flame (a slightly reducing flame was used) was approximately parallel to the axis of the joint being soldered. (See Figure 12.) By directing the flame in this manner, the material in the vicinity of the torch tip was heated to the soldering temperature,

---

\* A Borax and Boric Acid mixture.

The model was made consisting of two end  
 wide-flange sections. In joining the  
 the flame, the torch was held in a position  
 so that the flame (a slightly reducing flame  
 was used) was approximately parallel to the  
 axis of the joint being soldered. (See Figure  
 12.) By directing the flame in this manner,  
 the material in the vicinity of the torch  
 tip was heated to the soldering temperature.

while the material away from the torch tip in the direction of the flame became preheated to a relatively high temperature. When the correct temperature was reached, as indicated by the fluid flux, the silver solder rod was touched to the joint. The solder flowed freely along the joint until the joint became too cool. By moving the torch slowly and applying solder from the rod at about every inch, a strong joint was obtained throughout the length of the pieces.

If the joint is dirty, or if the flux is rubbed off at a point along the joint, no amount of heating will cause the solder to adhere to the pieces. In this event, wait until the pieces cool, clean and reflux the spot, then reheat and solder it.

### (3) Test samples and results

In order to check the strength of the silver solder joint in shear and tension, test samples of joints were prepared and tested. (See Figure 13.)

#### Shear test

a = 1 inch  
L = 1 inch  
b = 3/4 inch  
h = .056 inch

#### Tension test

a = 1 inch  
b = 1 inch  
h = .056 inch

Two inches of joint tested in shear was

... the joint is slightly, or it may be in  
 ... at a point along the joint, or  
 ... amount of loading, will cause the solder to  
 ... to the stress. In this event, wait  
 ... until the solder cools, then the relief the  
 ... then relief and solder.

(2) Test as shown and loaded.

In order to check the strength of the  
 silver solder joint in shear and tension,  
 test samples of joints were prepared and  
 tested. (See Figure 15.)

| Tension test  | Shear test    |
|---------------|---------------|
| a = 1 inch    | a = 1 inch    |
| b = 1 inch    | b = 1 inch    |
| h = .050 inch | h = 3/4 inch  |
|               | n = .050 inch |

Two inches of joint tested in shear was



stronger than the parent metal, while one inch tested in tension broke at 2830 pounds. The strength of the joint was seen to be more than sufficient for our purposes.

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## (4) Beam tests and results

## Beam #12

Beam #12 was a "T" beam constructed by using silver solder rod and an oxyactelylene torch, in Jig #2. It was tested on the vertical loading frame with the results as given below:

|                    |                 |
|--------------------|-----------------|
| Dimensions of beam | b = 1.5 inches  |
| (See Figure 10)    | t = .056 inches |
|                    | d = 1.56 inches |
|                    | L = 22 inches   |

The neutral axis was computed to be .417 inches above the base.

Moment of Inertia (I) = .0905

Deflection (D) = .0817 P x 10<sup>-3</sup>

Stress (f) = 69.3 P

## Dial Reading

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 10          | .4107       | .4132         | .0015            | .00082            | 83.0           | 690.0         |
| 25          | .4110       | .4141         | .0031            | .00204            | 52.0           | 1735.0        |
| 50          | .4110       | .4170         | .0060            | .00408            | 47.0           | 3470.0        |
| 75          | .4107       | .4186         | .0089            | .00612            | 45.5           | 5200.0        |
| 100         | .4108       | .4220         | .0112            | .00817            | 37.1           | 6930.0        |
| 125         | .4110       | .4242         | .0132            | .01020            | 29.4           | 8670.0        |
| 150         | .4111       | .4259         | .0148            | .01223            | 21.0           | 10400.0       |

This beam was distorted from heating. The web was not exactly centered on the flange. The joint, however, appeared to be very good.

to be very good. exactly centered on the flange. The joint, however, appeared This beam was distorted from handling. The weld was not

| Load | Zero  | Loaded | Ass. Def. | Comp. Def. | Diff. | Stress  |
|------|-------|--------|-----------|------------|-------|---------|
| 10   | .4107 | .4152  | .0015     | .0002      | .0013 | 220.1   |
| 25   | .4110 | .4141  | .0031     | .0005      | .0026 | 1428.0  |
| 50   | .4110 | .4140  | .0030     | .0008      | .0022 | 2470.0  |
| 75   | .4107 | .4186  | .0089     | .0012      | .0077 | 3200.0  |
| 100  | .4109 | .4230  | .0112     | .0017      | .0095 | 5010.0  |
| 125  | .4110 | .4242  | .0132     | .0100      | .0032 | 5870.0  |
| 150  | .4111 | .4259  | .0148     | .0122      | .0026 | 10400.0 |

Stress (1) = 81.3  
 Deflection (1) = .0017 x 10<sup>-3</sup>  
 Moment of Inertia (I) = .003

the base. The bonded area was assumed to be .415 inches above

Given:  
 Inertia (I) = .003  
 Stress (1) = 81.3  
 Deflection (1) = .0017 x 10<sup>-3</sup>

sof of 100 x 10<sup>-3</sup> x 10<sup>-3</sup> = 10<sup>-6</sup> in<sup>4</sup>  
 tested at 100 x 10<sup>-3</sup> x 10<sup>-3</sup> = 10<sup>-6</sup> in<sup>4</sup>  
 Given:

b. Furnace Brazing (See Figure 16)

In an effort to overcome the distortion of the material being joined resulting from localized heating with a torch, furnace brazing was tried. The bonding alloy was silver solder, in the form of a thin foil or sheet 0.005 inches thick. The pieces to be joined were prepared as stated in section III-A-1. Flux was applied to the surfaces for the purpose previously stated. Finally, strips of the foil were inserted in the joint between the pieces to be united, and the whole assembly clamped rigidly together. The pieces could be clamped rigidly together since there would be no differential expansion between the model components while in the furnace. The assembly was then inserted in the furnace.

(1) The furnace

The furnace used was a Lindberg type, belonging to the Metallurgy Department. It was an automatically controlled, electric furnace, equipped with a blower for circulating the air within it. Prior to inserting the model, the temperature was raised to 1175° F.

The model was left in the furnace for 15 minutes at the 1175° temperature, then

195815 02 (1)

the model, the two trustees were named in the deed of gift. The trustee was named in the deed of gift, and the trustee was named in the deed of gift.

0511

15 minutes to the 11:50 departure time.

Furnace Brazed Steel Beam

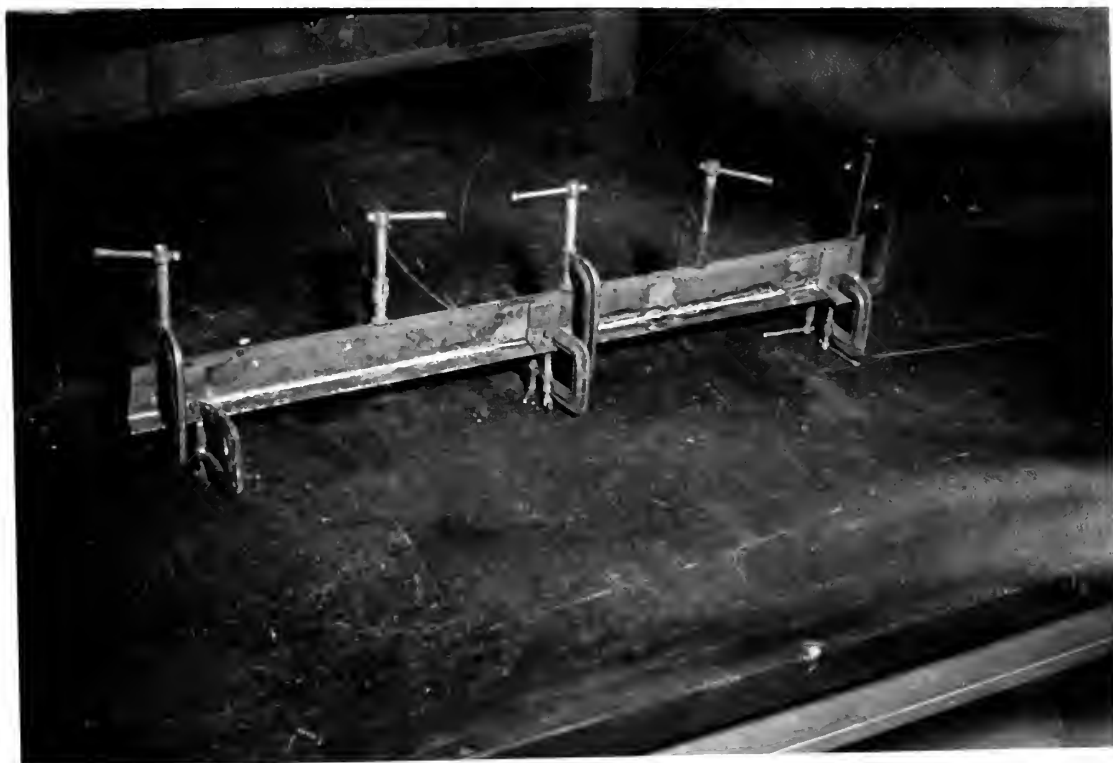


Figure 16





taken out and allowed to cool in air.

(2) Patching the model

The model referred to in the above paragraph was a wide-flange section, about 22 inches long. Near one end, the flange was not joined to the web for a distance of about 2 inches. It is presumed that this bad joint was caused by an uneven web which allowed too large an opening to be filled by the solder. This spot was patched by placing another strip of foil in the opening, fluxing it, and reheating the area with a torch.

See following pages for the results of the testing of these models.



taken out and allowed to cool in air.

(2) Patching the model

The model referred to in the above paragraph was a wide-flange section, about 22 inches long. Near one end, the flange was not joined to the web for a distance of about 2 inches. It is presumed that this bad joint was caused by an uneven web which allowed too large an opening to be filled by the solder. This spot was patched by placing another strip of foil in the opening, fluxing it, and reheating the area with a torch.

See following pages for the results of the testing of these models.



## (3) Beam test and results

## Beam #13

Beam #13 was a "T" beam fabricated by furnace brazing with silver solder using Jig #3 modified, and clamping the pieces rigidly together with "C" clamps. It was tested on the vertical loading frame with the following results.

|                    |          |        |
|--------------------|----------|--------|
| Dimensions of Beam | b = 1.5  | inches |
| (See Figure 10)    | t = .056 | inches |
|                    | d = 1.56 | inches |
|                    | L = 16   | inches |

The neutral axis was computed to be .417 inches above the base.

Moment of Inertia (I) = .0905

Deflection (D) = .0315 P x 10<sup>-3</sup>

Stress (f) = 50.4 P

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 10          | .1520       | .1476         | .0044            | .00315            | 28.5           | 504.0         |
| 35          | .1581       | .1478         | .0103            | .0110             | 6.8            | 1765.0        |
| 60          | .1600       | .1478         | .0122            | .0890             | 55.0           | 3030.0        |

The erratic results were thought to have been caused by buckling of the web as the load was applied. The beam showed very little distortion, and the joint appeared sound.

The neutral axis for the beam is at the center of the web. The moment of inertia of the web is  $I_w = 1.0 \times 10^{-6}$  in<sup>4</sup>. The moment of inertia of the flanges is  $I_f = 1.0 \times 10^{-6}$  in<sup>4</sup>. The total moment of inertia of the beam is  $I = 2.0 \times 10^{-6}$  in<sup>4</sup>. The deflection of the beam is  $\delta = 1.0 \times 10^{-3}$  in. The stress in the flanges is  $\sigma = 1.0 \times 10^4$  psi. The stress in the web is  $\tau = 1.0 \times 10^4$  psi.

The neutral axis for the beam is at the center of the web. The moment of inertia of the web is  $I_w = 1.0 \times 10^{-6}$  in<sup>4</sup>. The moment of inertia of the flanges is  $I_f = 1.0 \times 10^{-6}$  in<sup>4</sup>. The total moment of inertia of the beam is  $I = 2.0 \times 10^{-6}$  in<sup>4</sup>. The deflection of the beam is  $\delta = 1.0 \times 10^{-3}$  in. The stress in the flanges is  $\sigma = 1.0 \times 10^4$  psi. The stress in the web is  $\tau = 1.0 \times 10^4$  psi.

Moment of Inertia ( $I$ ) =  $2.0 \times 10^{-6}$  in<sup>4</sup>  
 Deflection ( $\delta$ ) =  $1.0 \times 10^{-3}$  in  
 Stress ( $\sigma$ ) =  $1.0 \times 10^4$  psi

| Load | Zero  | Deflection | Stress | Deflection | Stress |
|------|-------|------------|--------|------------|--------|
| 10   | 1.000 | 1.470      | .0044  | .0015      | 58.0   |
| 35   | 1.001 | 1.470      | .0103  | .0110      | 8.8    |
| 60   | 1.000 | 1.470      | .0188  | .0880      | 34.0   |

The elastic results were thought to have been caused by buckling of the web as the load was applied. The beam showed very little distortion, and the joint appeared sound.

## Beam #14

Beam #14 was a wide flange section which was furnace brazed using silver solder. Jigging method #3 modified, with the assembly clamped rigidly together, was used.

Dimensions of Beam  
(See Figure 10)

b = 1.5 inches  
t = .056 inches  
d = 1.61 inches  
L = 22 inches

Moment of Inertia (I) = .1172

Deflection (D) = .0631 P x 10<sup>-3</sup>

Stress (f) = 37.8 P

| <u>Load</u> | <u>Zero</u> | <u>Loaded</u> | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|-------------|---------------|------------------|-------------------|----------------|---------------|
| 110         | .18440      | .19490        | .0150            | .00695            | 33.8           | 4160.0        |
| 185         | .18450      | .20180        | .01730           | .01169            | 32.0           | 6990.0        |
| 60          | .18530      | .19130        | .00600           | .00380            | 36.5           | 2270.0        |

This beam appeared to be distortion free and unwarped throughout its length. The joint, after it was patched, appeared to be satisfactory. The cause of the bad test results could be attributed only to the imperfect joint, which, even after being patched, probably was not strong enough.

The beam appeared to be dislocated free and unimpeded  
 throughout its length. The joint, when it was released,  
 appeared to be satisfactory. The cause of the bad test  
 results could be attributed only to the imperfect joint,  
 which, even after being refitted, probably was not strong  
 enough.

Deflection (u) = 0.017  
 Deflection (v) = 0.017  
 Deflection (w) = 0.017

| Load | Zero   | Loaded | Act. def. | Calc. def. | Ratio |
|------|--------|--------|-----------|------------|-------|
| 110  | 1.1440 | 1.1440 | 0.010     | 0.0080     | 0.80  |
| 185  | 1.1440 | 1.1440 | 0.010     | 0.0110     | 0.91  |
| 60   | 1.1230 | 1.1230 | 0.0090    | 0.0070     | 0.78  |

This beam appeared to be dislocated free and unimpeded  
 throughout its length. The joint, when it was released,  
 appeared to be satisfactory. The cause of the bad test  
 results could be attributed only to the imperfect joint,  
 which, even after being refitted, probably was not strong  
 enough.



Types of Beams Constructed



Figure 17



#### IV. Check of Beam #11 by Electric Strain Gages

Beam #11 was found to be very satisfactory when loaded on the vertical loading frame. The average variation between the computed and actual deflections of this model under load was less than two percent. It could be presumed, then, that as a whole the model was acting as a wide-flange beam should. However, in order to check this beam further and in particular to find out something about the stress distribution at various sections along its length, several SR-4 electric strain gages were mounted on it.

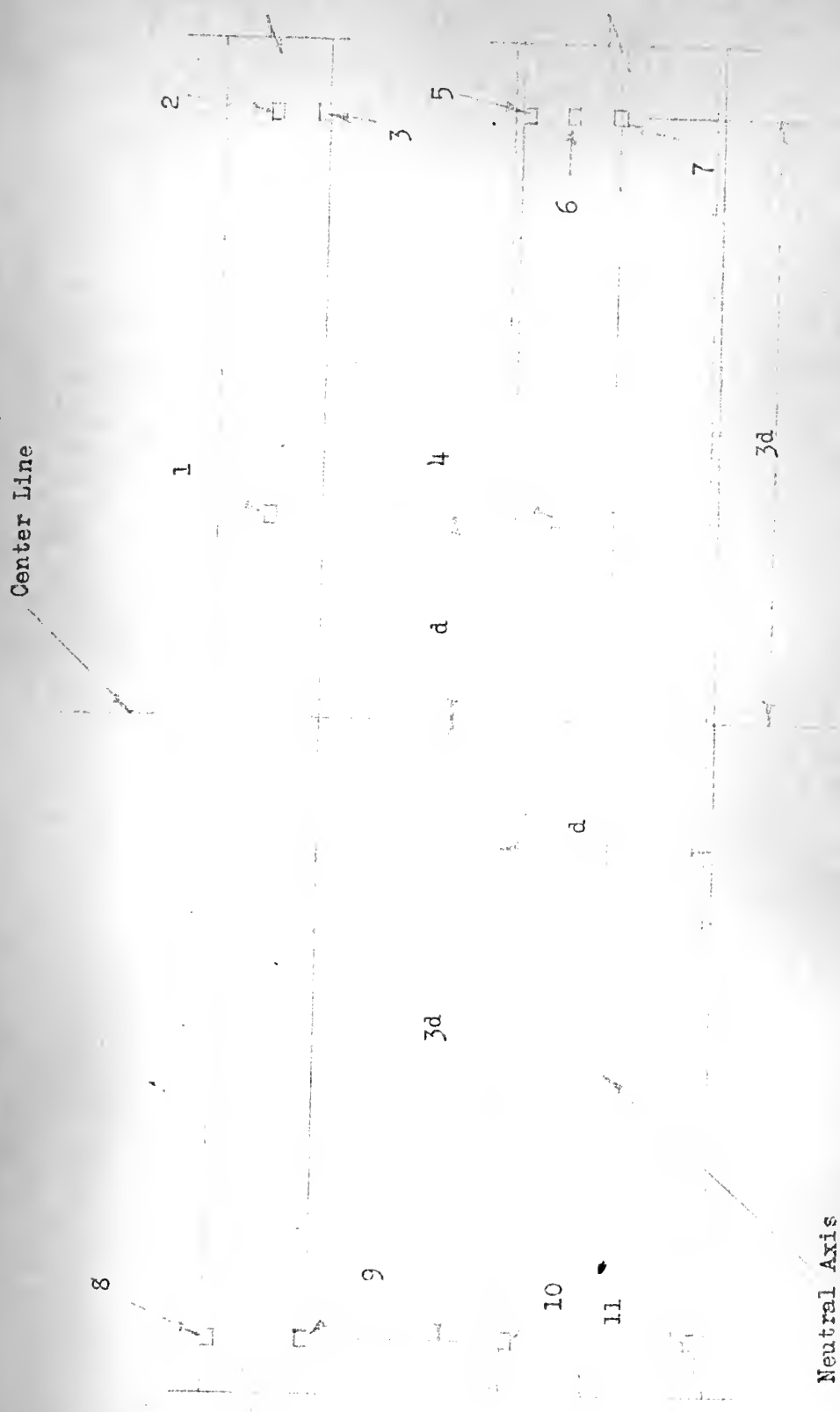
##### A. Location of Gages

A total of 11 gages were mounted on the web and flange of the beam as shown in Figure 18. Gages #1 and #4 were located at a distance of one beam depth away from the centerline of the beam, where the load was applied. According to the St. Venant principle, the stress at this section should be as given by the elastic theory. Gages #2, #3, #5, #6, and #7 were placed at a distance of three beam depths away from the load on one side of the mid-point while gages #8, #9, #10, and #11 were placed at a like distance on the other side of the mid-point. By locating the gages at these sections and placing some on the flange and others at different distances from the neutral axis on the web, we attempted to obtain a representative set of stress values.

##### B. Loading and Results

As stated previously, this beam had already been checked on the vertical loading frame. Since it was antici-





Alladin Soldered Beam # 11, Showing Strain Gage Location

Figure 18



pated that models made by our technique would be tested eventually on the horizontal loading frame, it was necessary first of all to check the action of the horizontal loading frame. (See Figure 19.) It was feared that there would be some friction losses caused by the change in direction of application of the load over a pulley.

Beam #11 was placed upon the loading frame in a horizontal position with steel ball bearings, sandwiched between glass plates, supporting it. (See Figure 14.) The same loading yoke that was used for vertical loading was supported at the center of the beam on ball bearings. The flanges at the end of the beam were pushed snugly against the vertical knife edge supports, making sure that the flanges were bearing along their whole length against the knife edges. A load of known value was applied, then, at the end of a steel cable, which passed over the pulley and was attached to the yoke. The results of this loading are shown below:

| <u>Load</u> | <u>Dial Reading</u> |               | <u>Act. Def.</u> | <u>Comp. Def.</u> | <u>% Diff.</u> | <u>Stress</u> |
|-------------|---------------------|---------------|------------------|-------------------|----------------|---------------|
|             | <u>Zero</u>         | <u>Loaded</u> |                  |                   |                |               |
| 10          | .0745               | .0826         | .0081            | .0080             | 1.25           | 405.0         |
| 20          | .0745               | .0900         | .0155            | .0160             | 3.1            | 810.0         |
| 30          | .0745               | .0979         | .0234            | .0240             | 2.5            | 1215.0        |
| 40          | .0745               | .1058         | .0313            | .0320             | 2.2            | 1620.0        |
| 50          | .0745               | .1138         | .0393            | .0400             | 1.75           | 2025.0        |

The results of this investigation are as follows:

1. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

2. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

3. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

4. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

5. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

6. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

7. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

8. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

9. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

10. The results of the investigation of the effect of the load on the rate of loading of the knife edge are as follows:

| Table 1 |       |       |       |       |      |       |
|---------|-------|-------|-------|-------|------|-------|
| Load    | Rate  | Rate  | Rate  | Rate  | Rate | Rate  |
| 10      | 0.045 | 0.085 | 0.090 | 0.090 | 1.50 | 0.045 |
| 20      | 0.045 | 0.090 | 0.090 | 0.090 | 2.1  | 0.045 |
| 30      | 0.045 | 0.090 | 0.090 | 0.090 | 2.1  | 0.045 |
| 40      | 0.045 | 0.090 | 0.090 | 0.090 | 2.1  | 0.045 |
| 50      | 0.045 | 0.090 | 0.090 | 0.090 | 2.1  | 0.045 |



# Horizontal Loading Frame



Figure 19



A comparison of the actual and computed deflections shows that the friction losses are negligible since the actual differences are no greater than those occurring with vertical loading. These encouraging results showed that the horizontal loading frame, with all of its advantages in accommodating large models and in the ease of applying diversified loads, could be used for future tests.

Leads from the SR-4 gages were connected with the indicating device, and values of strains read for different loads. The results of these loadings are shown on the next few pages.

The results of these investigations are shown in Table I. The results show that the rate of reaction is very low, and that the reaction is not very sensitive to the concentration of the reactants. The reaction is also not very sensitive to the temperature. The reaction is, however, very sensitive to the presence of a catalyst. The rate of reaction is increased by a factor of 10 when a catalyst is present. The catalyst is a substance which is not consumed in the reaction, but which speeds up the reaction. The catalyst is a substance which is not consumed in the reaction, but which speeds up the reaction. The catalyst is a substance which is not consumed in the reaction, but which speeds up the reaction.

Form of Computations for Stresses at Various Sections Along  
Beam #11

Computations:

Section at "d" distance from the center (See Figure 18)

$$M = \left(\frac{P}{2}\right) (24.5) = 12.25 P \quad I = .4128$$

$$\text{Computed } f = \frac{Mc}{I} = \frac{(12.25 P)(c)}{.4128} = 29.6 P c$$

$$\text{Actual } f = eE = e \times 10^{-7}$$

Section at 3 "d" distance from the center

$$M = \left(\frac{P}{2}\right) (19.56) = 9.78 P$$

$$\text{Computed } f = \frac{Mc}{I} = \frac{(9.78 P)(c)}{.4128} = 23.7 P c$$

$$\text{Actual } f = eE = e \times 10^{-7}$$

Terms Defined:

I, moment of inertia, inches<sup>4</sup>

M, bending moment, inch lbs.

P, load, lbs.

f, stress, lbs./inch<sup>2</sup>

c, distance from neutral axis of beam to center of gage

e, strain indicated by SR-4 gage, micro-inches/inch

E, modulus of elasticity of aluminum, lbs./inch<sup>2</sup>

Values of c:

|               |               |               |
|---------------|---------------|---------------|
| gage 1, 1.238 | gage 5, .88   | gage 9, 1.238 |
| gage 2, 1.238 | gage 6, .45   | gage 10, .88  |
| gage 3, 1.238 | gage 7, 0     | gage 11, .93  |
| gage 4, .47   | gage 8, 1.238 |               |

Values of c:

|               |               |               |
|---------------|---------------|---------------|
| Page 1, 1.238 | Page 2, .88   | Page 9, 1.238 |
| Page 2, 1.238 | Page 6, .48   | Page 10, .88  |
| Page 3, 1.238 | Page 7, 0     | Page 11, .88  |
| Page 4, .47   | Page 8, 1.238 |               |

E, modulus of elasticity of aluminum, lbs./in.<sup>2</sup>

e, strain indicated by 10-4 gage, micro-inches/inch

c, distance from neutral axis of beam to center of gage

f, stress, lbs./in.<sup>2</sup>

P, load, lbs.

M, bending moment, inch lbs.

I, moment of inertia, in.<sup>4</sup>

Terms Defined:

Volume  $V = A \times L = 107$

Corrugated  $T = \frac{W \times (L \times T \times C)}{L} = \frac{40 \times (18.75 \times 7/8)}{1} = 32.7$  lbs

$V = (\frac{1}{2}) (12.140) = 2.77$

Volume of "T" section = 107

Volume of "C" section = 107

Volume of "L" section = 107

$V = (\frac{1}{2}) (12.140) = 2.77$

CO, 100, 000

P = 10 lbs.

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> | <u>11</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|-----------|
| Zero        | 0622     | 0113     | 0732     | 1900     | 0370     | 1361     | 1453     | 1052     | 1884     | 0798      | 0260      |
| Loaded      | 0598     | 0091     | 0709     | 1887     | 0350     | 1351     | 1452     | 1022     | 1860     | 0779      | 0273      |
| e           | 24       | 22       | 23       | 13       | 20       | 10       | 1        | 30       | 24       | 19        | 13        |
| Act. f      | 240      | 220      | 230      | 130      | 200      | 100      | 10       | 300      | 240      | 190       | 130       |
| Comp. f     | 367      | 293      | 293      | 138      | 207      | 106      | 0        | 293      | 293      | 209       | 220       |
| % Diff.     | 34.5     | 24.9     | 21.5     | 5.8      | 3.4      | 5.7      |          | 2.4      | 18.1     | 9.0       | 41.0      |

P = 20 lbs.

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> | <u>11</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|-----------|
| Zero        | 0622     | 0113     | 0732     | 1900     | 0370     | 1361     | 1453     | 1052     | 1884     | 0798      | 0260      |
| Loaded      | 0561     | 0064     | 0686     | 1871     | 0332     | 1340     | 1454     | 0998     | 1835     | 0761      | 0288      |
| e           | 61       | 49       | 46       | 29       | 38       | 21       | 1        | 54       | 49       | 37        | 28        |
| Act. f      | 610      | 490      | 460      | 290      | 380      | 210      | 10       | 540      | 490      | 370       | 280       |
| Comp. f     | 735      | 587      | 587      | 277      | 415      | 213      | 0        | 588      | 588      | 417       | 441       |
| % Diff.     | 17.0     | 16.5     | 21.7     | 4.7      | 8.5      | 1.4      |          | 8.2      | 16.7     | 11.2      | 36.4      |

P = 30 lbs.

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> | <u>11</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|-----------|
| Zero        | 0622     | 0113     | 0732     | 1900     | 0370     | 1361     | 1455     | 1052     | 1884     | 0798      | 0260      |
| Loaded      | 0529     | 0035     | 0662     | 1858     | 0312     | 1328     | 1453     | 0968     | 1808     | 0743      | 0300      |
| e           | 93       | 78       | 70       | 42       | 56       | 33       | 2        | 84       | 76       | 55        | 40        |
| Act. f      | 930      | 780      | 700      | 420      | 580      | 330      | 20       | 840      | 760      | 550       | 400       |
| Comp. f     | 1100     | 880      | 880      | 417      | 625      | 320      | 0        | 881      | 881      | 626       | 662       |
| % Diff.     | 15.5     | 11.3     | 13.6     | 0.7      | 7.2      | 3.1      |          | 4.6      | 13.7     | 12.1      | 39.7      |

| Grade   | 1    | 2    | 3    | 4    | 5    | 6    | 7    | 8    | 9    | 10   | 11   |
|---------|------|------|------|------|------|------|------|------|------|------|------|
| Zero    | 0825 | 0118 | 0738 | 1800 | 0870 | 1881 | 1455 | 1083 | 1884 | 0738 | 0825 |
| Loaded  | 0825 | 0038 | 0888 | 1888 | 0818 | 1888 | 1458 | 0888 | 1888 | 0748 | 0825 |
| •       | 81   | 78   | 78   | 70   | 48   | 82   | 3    | 84   | 78   | 88   | 40   |
| Act. 1  | 810  | 480  | 480  | 480  | 880  | 880  | 880  | 880  | 480  | 880  | 880  |
| Comp. 1 | 788  | 887  | 887  | 887  | 418  | 818  | 0    | 887  | 887  | 887  | 887  |
| % Diff. | 17.0 | 18.8 | 21.7 | 4.7  | 8.8  | 1.4  |      | 4.8  | 18.7 | 18.7 | 20.7 |

| Grade   | 1    | 2    | 3    | 4    | 5    | 6    | 7    | 8    | 9    | 10   | 11   |
|---------|------|------|------|------|------|------|------|------|------|------|------|
| Zero    | 0825 | 0118 | 0738 | 1800 | 0870 | 1881 | 1455 | 1083 | 1884 | 0738 | 0825 |
| Loaded  | 0825 | 0038 | 0888 | 1888 | 0818 | 1888 | 1458 | 0888 | 1888 | 0748 | 0825 |
| •       | 81   | 78   | 78   | 70   | 48   | 82   | 3    | 84   | 78   | 88   | 40   |
| Act. 1  | 810  | 480  | 480  | 480  | 880  | 880  | 880  | 880  | 480  | 880  | 880  |
| Comp. 1 | 788  | 887  | 887  | 887  | 418  | 818  | 0    | 887  | 887  | 887  | 887  |
| % Diff. | 17.0 | 18.8 | 21.7 | 4.7  | 8.8  | 1.4  |      | 4.8  | 18.7 | 18.7 | 20.7 |

| Grade   | 1    | 2    | 3    | 4    | 5    | 6    | 7    | 8    | 9    | 10   | 11   |
|---------|------|------|------|------|------|------|------|------|------|------|------|
| Zero    | 0825 | 0118 | 0738 | 1800 | 0870 | 1881 | 1455 | 1083 | 1884 | 0738 | 0825 |
| Loaded  | 0825 | 0038 | 0888 | 1888 | 0818 | 1888 | 1458 | 0888 | 1888 | 0748 | 0825 |
| •       | 81   | 78   | 78   | 70   | 48   | 82   | 3    | 84   | 78   | 88   | 40   |
| Act. 1  | 810  | 480  | 480  | 480  | 880  | 880  | 880  | 880  | 480  | 880  | 880  |
| Comp. 1 | 788  | 887  | 887  | 887  | 418  | 818  | 0    | 887  | 887  | 887  | 887  |
| % Diff. | 18.8 | 18.8 | 18.8 | 0.1  | 8.8  | 3.1  |      | 4.8  | 18.7 | 18.7 | 20.7 |



$P = 40 \text{ lbs.}$

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> | <u>11</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|-----------|
| Zero        | 0622     | 0113     | 0732     | 1900     | 0370     | 1361     | 1453     | 1052     | 1884     | 0798      | 0260      |
| Loaded      | 0498     | 0009     | 0640     | 1840     | 0292     | 1317     | 1452     | 1022     | 1860     | 0779      | 0273      |
| e           | 124      | 104      | 92       | 60       | 78       | 44       | 1        | 30       | 24       | 19        | 13        |
| Act. f      | 1240     | 1040     | 920      | 600      | 780      | 440      | 10       | 300      | 240      | 190       | 130       |
| Comp. f     | 1470     | 1175     | 1175     | 555      | 831      | 426      | 0        | 293      | 293      | 209       | 220       |
| % Diff.     | 15.6     | 11.5     | 21.7     | 8.1      | 6.1      | 3.3      |          | 2.4      | 18.1     | 9.0       | 41.0      |

$P = 50 \text{ lbs.}$

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> | <u>11</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|-----------|
| Zero        | 0624     | 2115     | 0731     | 1900     | 0364     | 1355     | 1451     | 1052     | 1884     | 0798      | 0260      |
| Loaded      | 0461     | 1984     | 0620     | 1829     | 0273     | 1301     | 1449     | 0906     | 1763     | 0709      | 0329      |
| e           | 163      | 131      | 111      | 71       | 91       | 54       | 2        | 146      | 121      | 89        | 69        |
| Act. f      | 1630     | 1310     | 1110     | 710      | 910      | 540      | 20       | 1460     | 1210     | 890       | 690       |
| Comp. f     | 1838     | 1470     | 1470     | 695      | 1040     | 533      | 0        | 1470     | 1470     | 1442      | 1102      |
| % Diff.     | 11.3     | 10.9     | 24.5     | 2.2      | 12.5     | 1.3      |          | 0.7      | 17.6     | 14.6      | 37.5      |



### C. Conclusions

In general, the difference between the computed and actual values of stress becomes less as the load is increased. This indicates that for higher stresses, errors introduced by slight inaccuracies in construction have reduced effect. The difference between values of stress indicated by gages 2 and 3 shows that the edge of the flange participates less in resisting the load than the center of the flange. This was probably due to a slight buckling of the flange at its outer edge. Gages 4 and 6, which were located on the web midway between the neutral axis and the flange, gave consistently good results. This was due, it was thought, to their location away from the point where the web and flange were joined. Gages 5 and 10, located on the web near the flange, gave good results, but a little less accurately than gages 4 and 6. The difference between stresses at gages 8 and 9 was caused by the knife edge of the loading yoke not bearing evenly across the top flange. This caused one side of the flange to assume more load than the other. No reason can be given for the large discrepancy between the computed and actual stresses given by gage 11, unless it was due to a defective gage.

An overall comparison of computed and actual stresses indicated that the model was acting satisfactorily. The stress distribution closely approximated that given by the flexural theory.



## V. Constructing and Testing a Rigid Frame

The construction and testing of a rigid frame was considered as the culmination of all the work done on this thesis. A rigid frame is one that is constructed to resist moment at the joints. The method of building a joint to resist moment may be either by riveting or welding. It is in this section that we discuss how we constructed and tested a welded rigid frame.

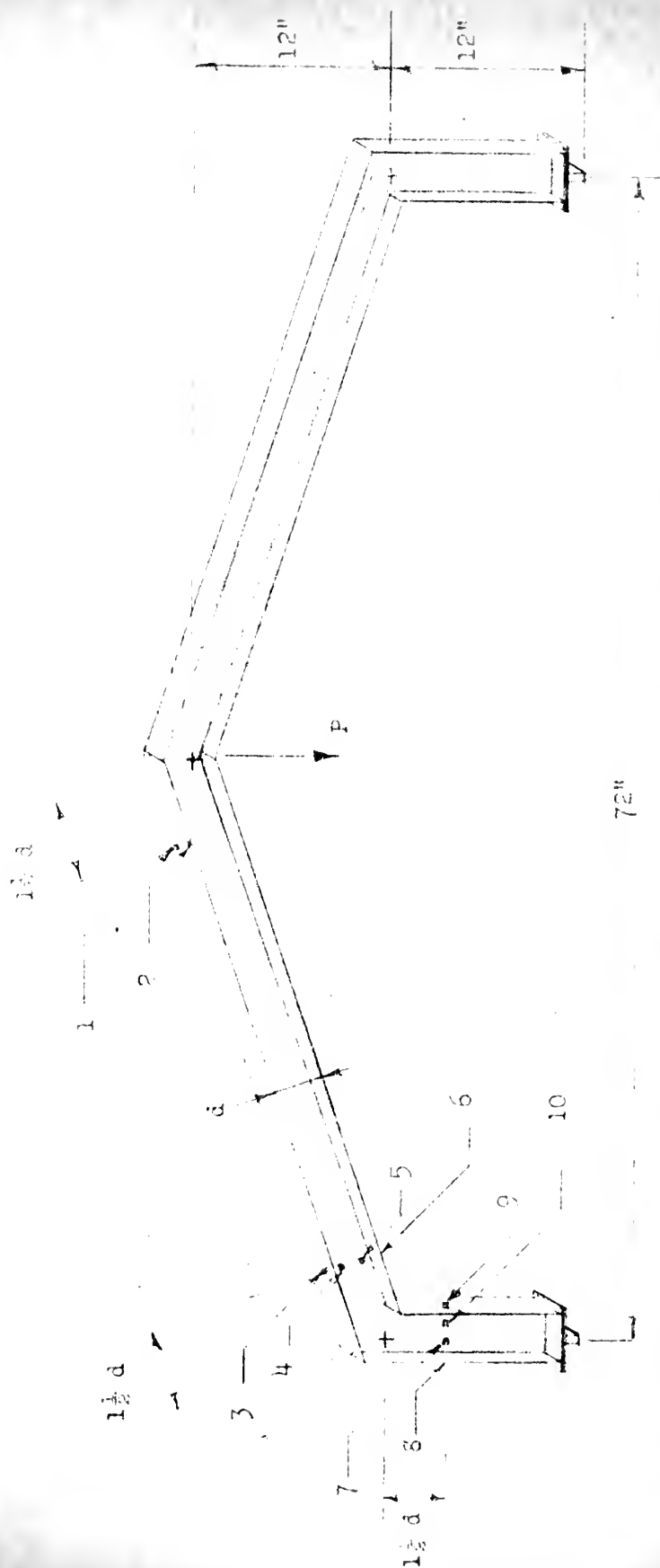
### A. Purpose

The task of constructing a rigid frame was undertaken for two main reasons. The first, and most important to us, was to investigate the soundness of our techniques and methods in building models other than plain straight beams. Our last tests of beams were very successful, however, the beams were all of the same design. Thus, in order to be certain that the techniques and methods were sound, we built the rigid frame as shown in Figure 20. It would have been possible to construct a differently shaped model to test, but it was for the reason mentioned below that made us decide in favor of a rigid frame. As evidenced by the tests run on rigid frames, as mentioned in the introduction, there was still much to be learned, particularly about the stresses at the knees. Therefore, by building a rigid frame, we hoped not only to prove that our techniques were sound, but also to advance, perhaps, the understanding of stresses at knees in rigid frames.

### B. Design

The design of the frame was not completely an





### Rigid Frame Showing Location of Strain Gages

Figure 20





arbitrary one. We attempted to build a frame that, although not an exact copy of a large structure, was similar in most ways to one that might possibly be built. The important feature that influenced our design was the limit we placed on the amount of horizontal shear that we would allow. Although our sample tests indicated that we could go to about 12 lbs./inch, we tried to stay down lower than 8 lbs./inch to be sure that no harm would be done to the welds. Therefore, we designed the frame to give us a maximum deflection with the span being used, along with the lowest possible horizontal shear for any given load.

### C. Construction

The rigid frame was constructed using the alladin solder method. It was held and supported as indicated in the discussion under jiggling #3 modified.

The base detail of the legs is indicated in Figure 21. Since there are several ways of testing the frame, it was necessary to develop a base detail that would accommodate any desired method. Therefore, a piece of aluminum plate about 1/4 inch thick was welded to the base of each leg. Two holes were drilled through each plate so that different types of base attachments could be used. The particular attachment we used was a simulated pin on each leg. This was accomplished by bolting a piece of steel bar, rounded on the bottom, to the plate. When the rigid frame was mounted on the horizontal loading frame, the steel bar was inserted into a slotted plate mounted on the loading frame. This slot supported the bar at the bottom and along the sides.

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### C. Construction

The rigid frame was constructed... solder method. It was... discussion under... The base... 21. Since there are several ways of testing the frame, it was necessary to develop a base detail that would accommodate any desired method. Therefore, a piece of aluminum plate about 1/4 inch thick was welded to the base of each leg. Two holes were drilled through each plate so that different types of base attachments could be used. The particular attachment we used was a simulated pin on each leg. This was accomplished by bolting a piece of steel bar, rounded on the bottom, to the plate. When the rigid frame was mounted on the horizontal loading frame, the steel bar was inserted into a slotted plate mounted on the loading frame. This slot supported the bar at the bottom and along the sides.

## Rigid Frame Base Details

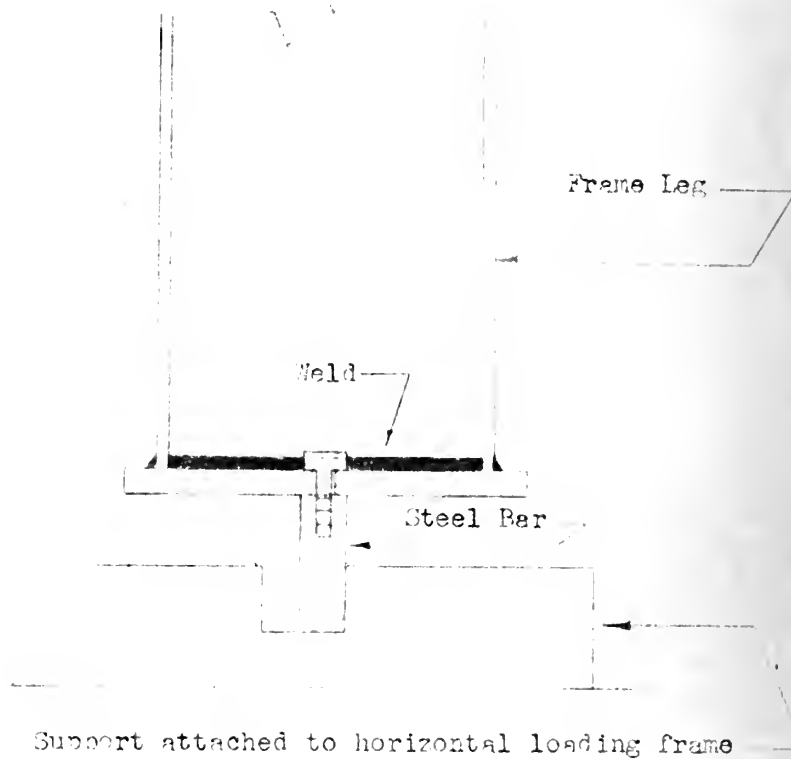


Figure 21



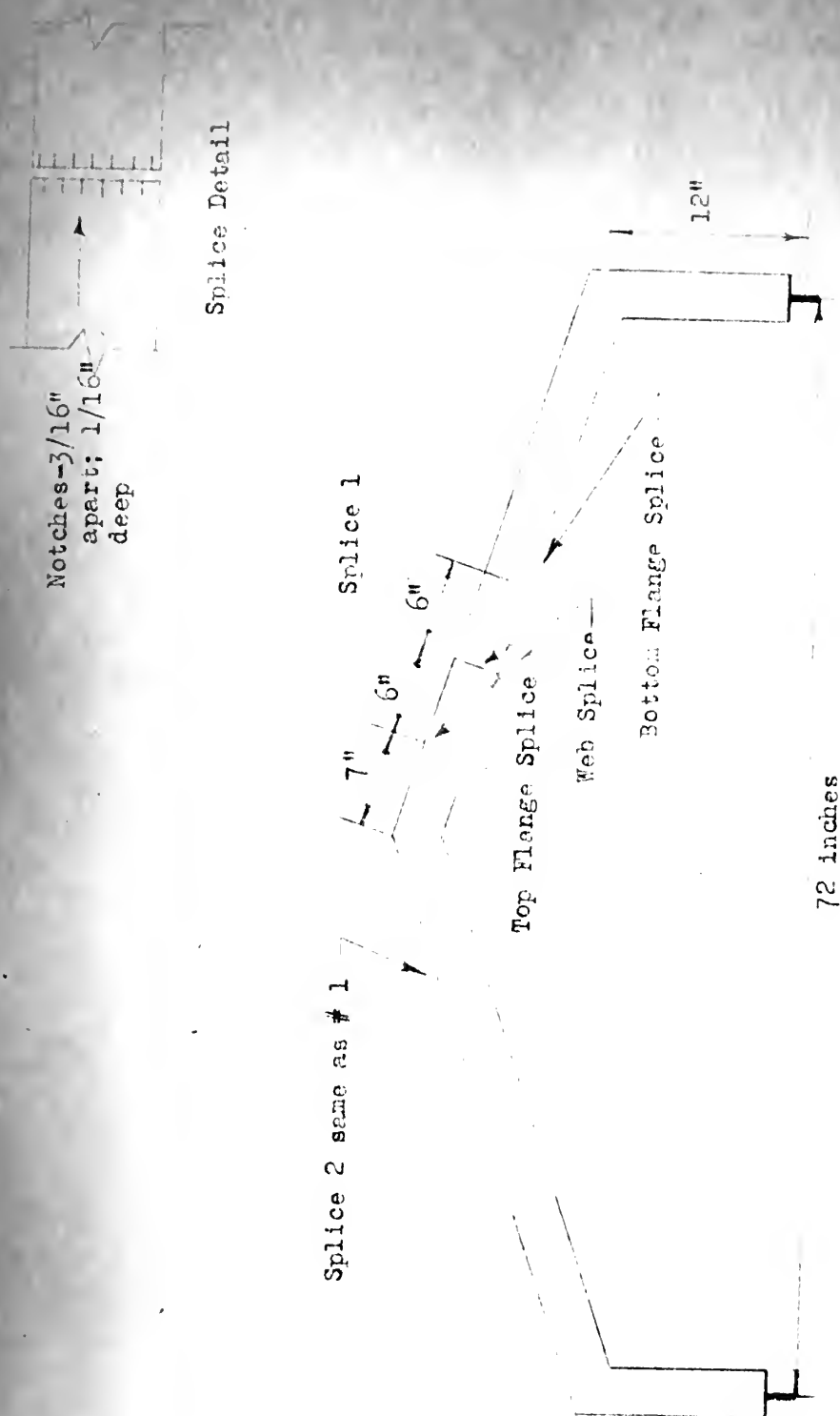
Since the length of the frame was 6 ft., it was impractical to cut the flanges and web in one piece. Therefore, it was necessary to devise a method for splicing. It was felt that the best way to insure the maximum strength was to stagger the splices. The location of these splices are shown in Figure 22. In making a splice, the ends of the material should be prepared as shown in Figure 22. This is a recognized method for butt joints as recommended by the Aluminum Company of America. The splices were made using alladin solder, care being used not to apply too much heat, such that the pieces being joined would warp at the splice.

#### D. Mounting

The frame was mounted on the horizontal loading frame in the same way that beam #11 was mounted. (See Figures 23, 24 and 25.) The four support points used were under the two knees, and one on each side of the load point. Lateral support was provided by two pound weights placed on the frame above the support points. It should be noted here that great care must be taken to insure that the beam is supported correctly at the bases. It is important to have both the bottom of the steel bar and the side of the steel bar bearing along the whole length of the support, or the readings taken will be inaccurate.

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will be inaccurate.



Rigid Frame showing location of Splices and Splice Detail.

Figure 22





Rigid Frame on Horizontal  
Loading Device

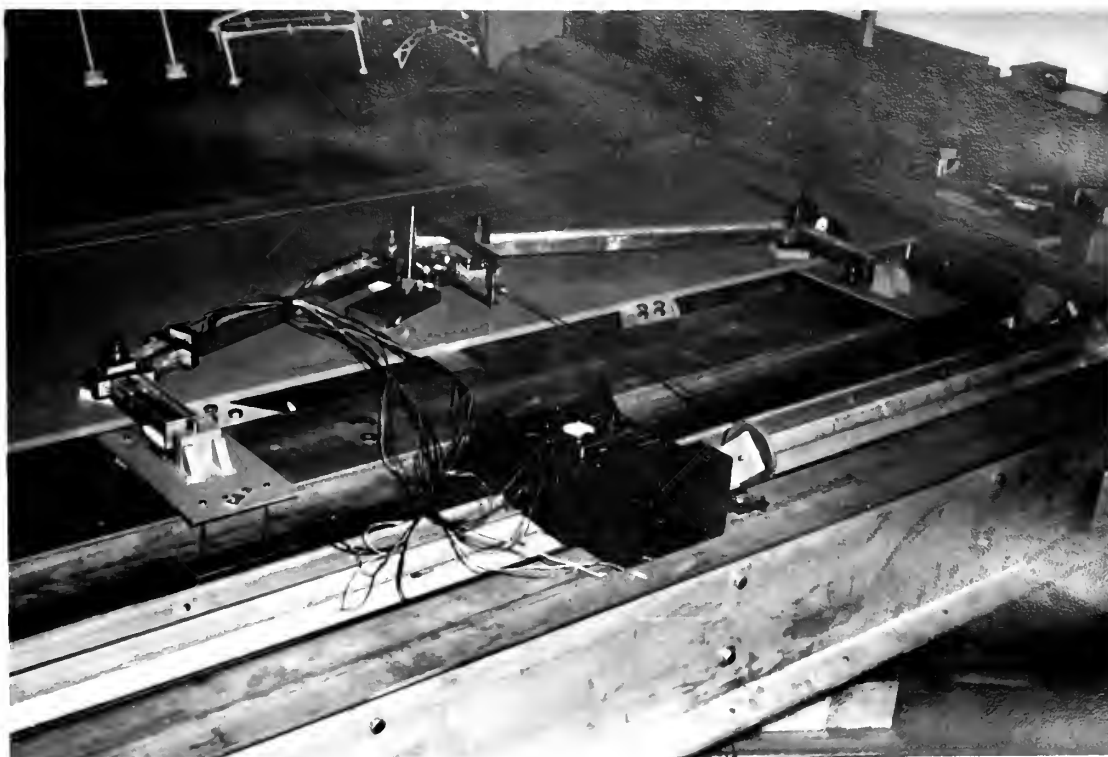


Figure 23



Rigid Frame Peak Detail

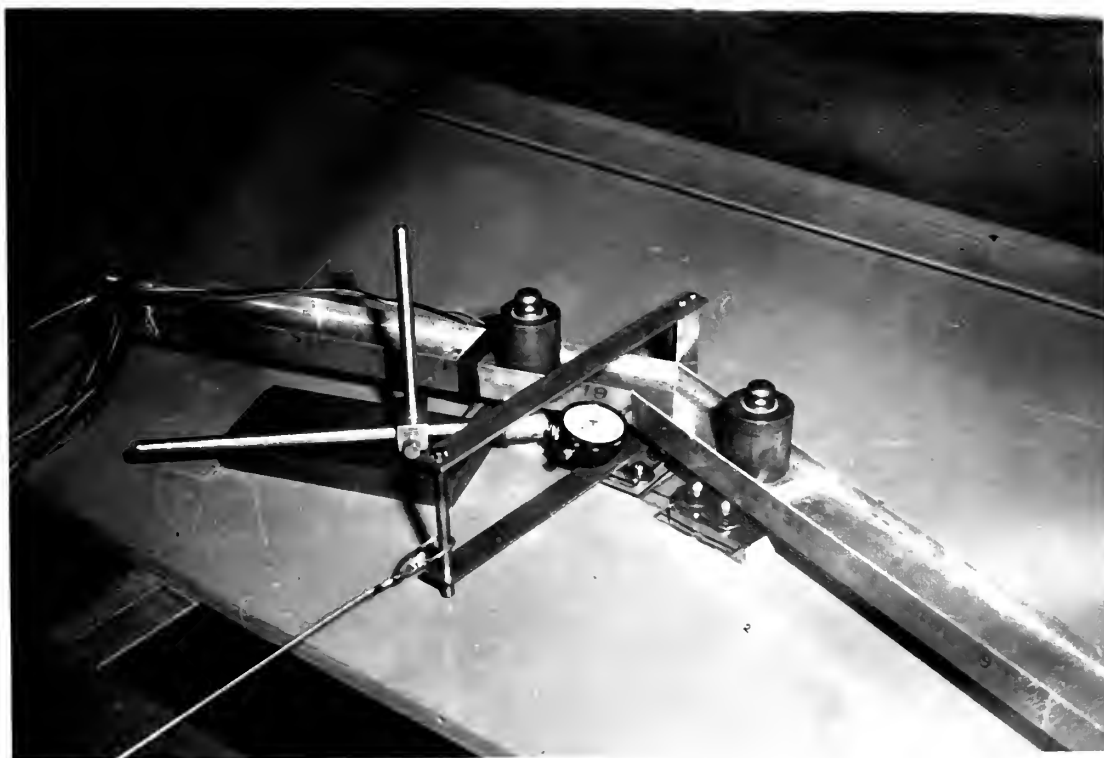


Figure 24



### E. Computations

#### 1. Solution for Deflection of Rigid Frame by the Conjugate Structure Method for a Pinned Base.

See Figure 26

$$(a) \quad D_A = M_{xx} = (20)(341 \text{ P})(2) = 13640 \text{ P}$$

$$(b) \quad d_H = M_{xx} = (72)(8)(2) + (2)(455)(18) + (2)(228)(20) = 26632$$

$$H_A = \frac{13640 \text{ P}}{26632} = .513 \text{ P}$$

$$(c) \quad (60.9 \text{ P})(6.25) + (65.75)(60.9 \text{ P}) + (37 \text{ P})(72) - (51.7 \text{ P})(30.25) - (51.7 \text{ P})(41.75) - 72 \theta_E = 0$$

$$\frac{3333 \text{ P}}{72} = \theta_E = 46.25 \text{ P}$$

$$\frac{EID_B}{2} = (46.25 \text{ P})(12) - (37.0 \text{ P})(4) + (60.9 \text{ P})(2.08) - (51.7 \text{ P})(8.16) = 111.8 \text{ P}$$

$$D_D = D_B = \frac{(2)(111.8)}{(10^7)(.4)} = .0000559 \text{ P}$$

$$EID_C = (46.25 \text{ P})(36) + (51.7 \text{ P})(5.75) - (60.9 \text{ P})(29.75) - (37 \text{ P})(36) = 1182 \text{ P}$$

$$D_C = \frac{1182 \text{ P}}{(10^7)(.4)} = .000296 \text{ P}$$



## Diagram for solution of Conjugate Structure

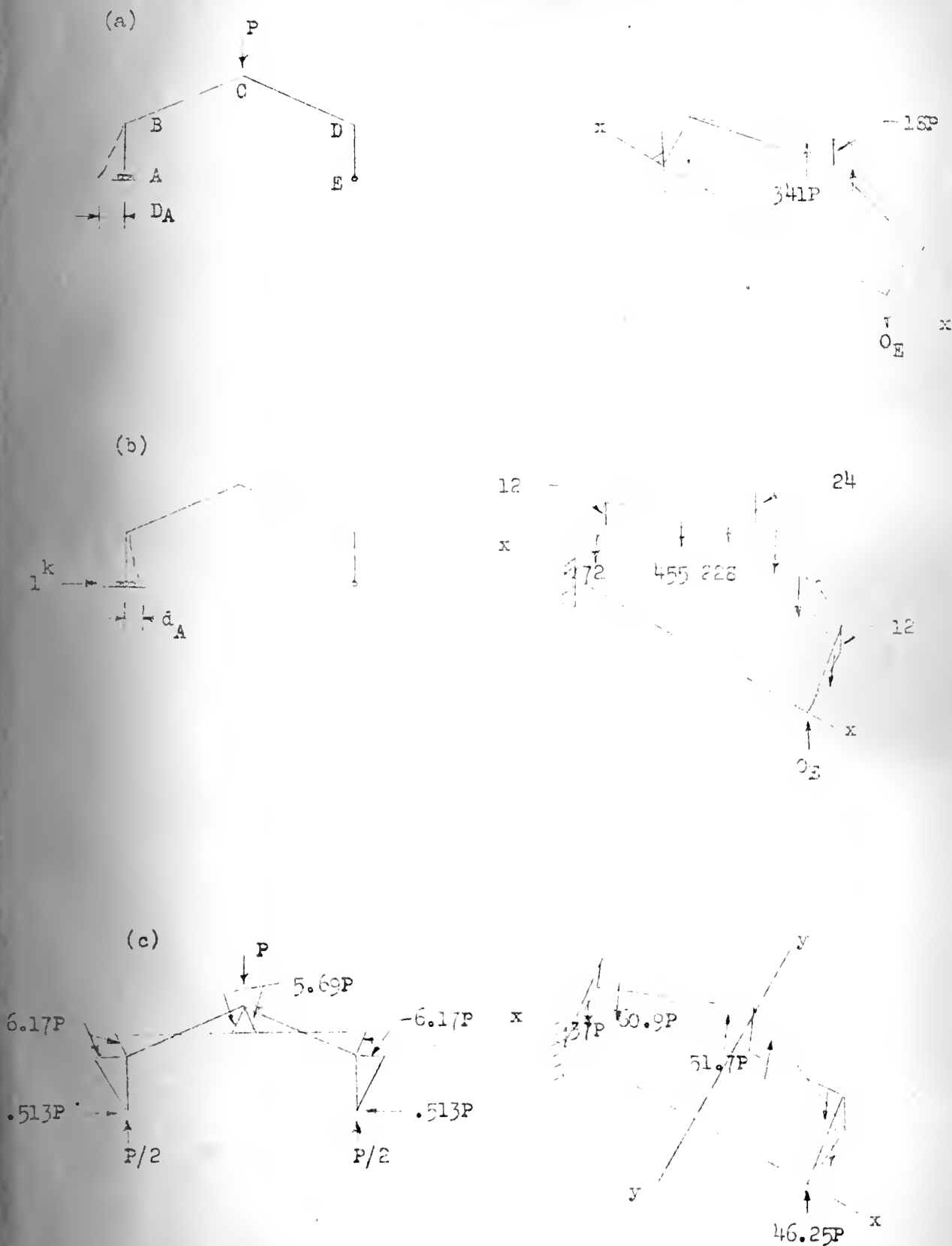


Figure 26





2. Check Solution for Deflections of Rigid Frame  
by Integration Using the Method of Virtual Work

$$EID_C = \int M m dx$$

$$EID_C = \int_0^{12} (.513 Px)(.513 x) dx$$

$$+ \int_0^{37.93} \left[ (.513 P)(12 + x \sin a) - \frac{P}{2} (x \cos a) \right] \left[ (.513)(12 + x \sin a) - .5 (x \cos a) \right]$$

$$= .264 P \int_0^{12} x^2 dx + \int_0^{37.93} (36.1 P - 3.85 P x + .0974 Px^2) dx$$

$$= 152 P + 1441 P - 2760 P + 1765 P$$

$$= 598 P$$

$$D_C = 2 \left( \frac{598 P}{EI} \right) = 2 \left( \frac{598 P}{(10^7)(.4)} \right) = .000299 P$$

$$f(x) = \frac{1}{x^2} = x^{-2}$$

$$f'(x) = -2x^{-3} = -\frac{2}{x^3}$$

$$f'(1) = -\frac{2}{1^3} = -2$$

$$f'(2) = -\frac{2}{2^3} = -\frac{1}{2}$$

$$\lim_{x \rightarrow 0} \frac{f(x) - f(0)}{x - 0} = \lim_{x \rightarrow 0} \frac{\frac{1}{x^2} - 0}{x} = \lim_{x \rightarrow 0} \frac{1}{x^3} = \infty$$

### 3. Corrections to Deflections Resulting from Movement of Base.

It was noted that the base deflected a slight amount when the structure was loaded; therefore, it was necessary to correct previously computed deflections for this movement. This was accomplished by interpolating between the deflections resulting from a pinned base and a base on rollers to obtain the correct deflections.

- a. Solution for horizontal deflection at E, with E on rollers. (See Figure 27.)

$$D_E = \int \frac{Mm}{EI} dx$$

$$\frac{EI}{2} D_E = \int_0^{37.93} (P/2)(.949)(x)(12 + .316x) dx = 6820 P$$

$$D_E = \frac{(2)(6820)(P)}{(.4)(10^7)} = .00341 P$$

- b. Solution for vertical deflection at C with E on rollers. (See Figure 27.)

$$D_C = \int \frac{Mm}{EI} dx$$

$$\frac{EI}{2} D_C = \int_0^{36} .25 P x^2 dx = 3888 P$$

$$D_C = \frac{(2)(3888 P)}{(.4)(10^7)} = .001944 P$$

- c. Sample correction for any load P.

|                   | $D_C$     | $D_E$    |
|-------------------|-----------|----------|
| Base pinned       | .000296 P | 0        |
| Actual Conditions | Z         | R        |
| Base on rollers   | .001944 P | .00341 P |

2. The data are as follows:

3. The data are as follows:

$$D_p = \sqrt{\frac{2 \pi \times 10^6}{1.5}}$$

$$\frac{D_p}{2} = \sqrt{\frac{2 \pi \times 10^6}{1.5}} = 27.95$$

$$D_p = \frac{(2)(27.95)}{(1.4)(10^6)} = .00341$$

4. The data are as follows:

$$D_p = \sqrt{\frac{2 \pi \times 10^6}{1.5}}$$

$$\frac{D_p}{2} = \sqrt{\frac{2 \pi \times 10^6}{1.5}} = 27.95$$

$$D_p = \frac{(2)(27.95)}{(1.4)(10^6)} = .00341$$

5. Sample correction for any load P.

| Base on rollers | Actual Conditions | Base pinned |
|-----------------|-------------------|-------------|
| .001944 P       | N                 | .000296 P   |
| .00341 P        | N                 | O           |

## Diagram for Solution of Deflections by Virtual work

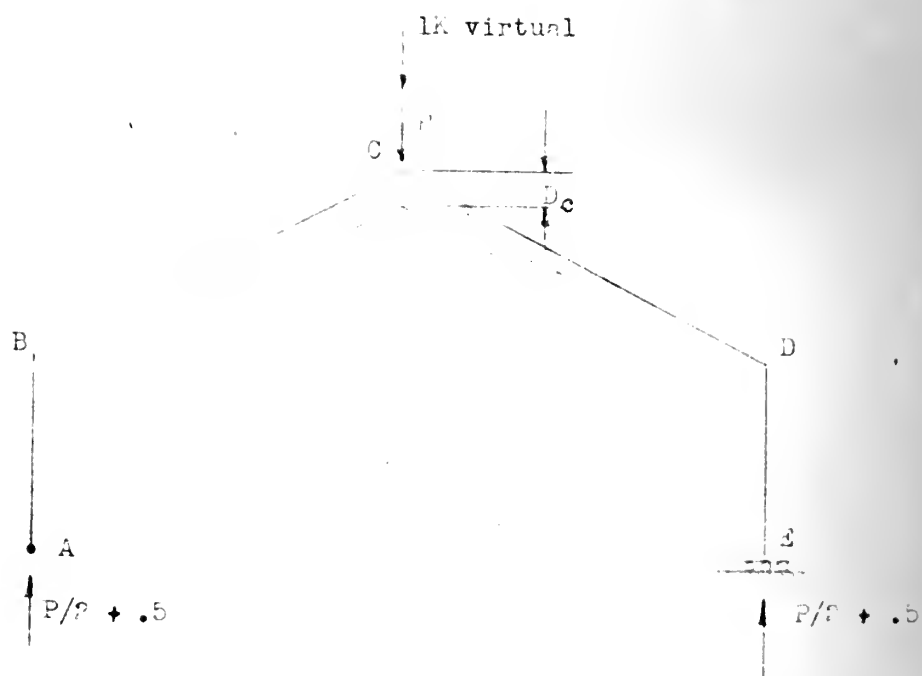
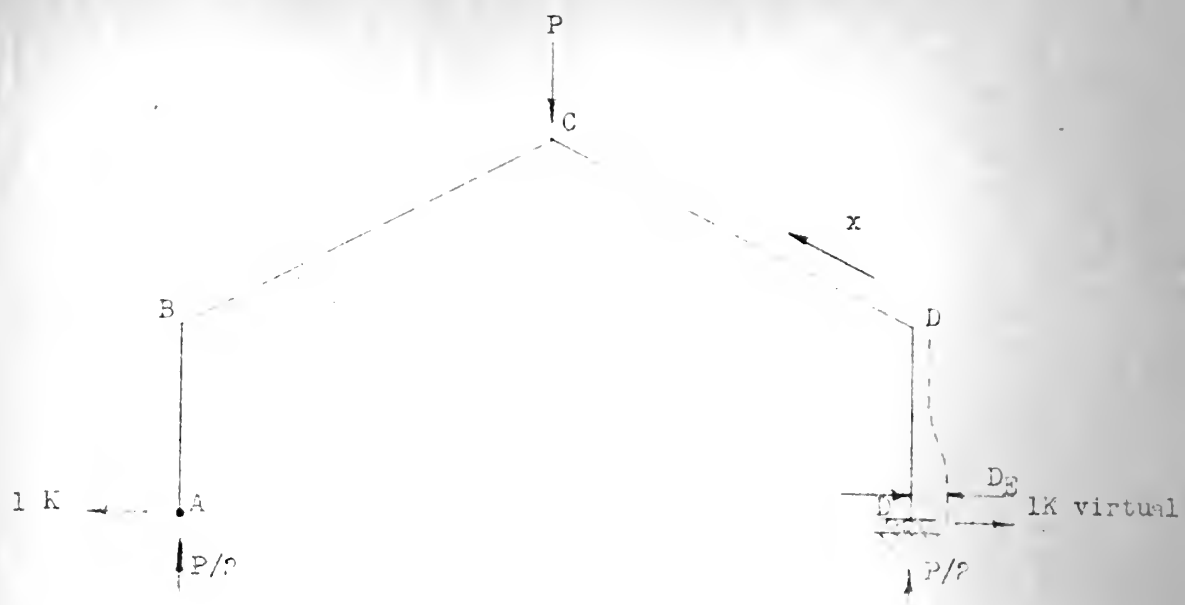


Figure 27



R is the sum of the outward deflections at the bases, as determined by mechanical dials.

Z is the corrected deflection at C, arrived at by interpolation.

The corrected deflection at the knee is arrived at by applying the movement of the base directly to the computed value at the knee.

4. Computations for stresses at various sections along the frame. (See Figure 20.)

On the leg, 1.5 d from knee;

$$M = (.513 P)(8.30) = 4.27 P$$

$$f = \frac{Mc}{I} = \frac{(4.27 P)(c)}{.40} = 10.66 Pc$$

On the girder, 1.5 d from knee

$$M = .513 P \left[ (3.75) \left( \frac{12}{37.93} \right) + 12 \right] - \left( \frac{P}{2} \right) (3.75) \left( \frac{36}{37.93} \right) = 4.99 P$$

$$f = \frac{Mc}{I} = \frac{(4.99 P)(c)}{.40} = 12.5 Pc$$

On the girder 1.5 d from C:

$$M = (.513 P) \left[ (34.55) \left( \frac{12}{37.93} \right) + 12 \right] - (P/2)(34.55) \left( \frac{36}{37.93} \right) = 4.60 P$$

$$f = \frac{Mc}{I} = \frac{(4.60 P)(c)}{.40} = 11.5 Pc$$

5. Correction to stresses resulting from movement of base.

Due to the movement of the base it was necessary to apply a correction to the stresses computed in section 4

1. The first step in the analysis of the data is to determine the average value of the stresses at each section. This is done by summing the stresses at each section and dividing by the number of sections.

$$\begin{aligned}
 \bar{\sigma} &= \frac{1}{n} \sum_{i=1}^n \sigma_i \\
 \bar{\sigma} &= \frac{1}{10} (10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00) \\
 \bar{\sigma} &= 10.00 \text{ psi}
 \end{aligned}$$

$$\begin{aligned}
 \text{On the other hand, } \bar{\sigma} &= 10.00 \text{ psi} \\
 \bar{\sigma} &= \frac{1}{n} \sum_{i=1}^n \sigma_i \\
 \bar{\sigma} &= \frac{1}{10} (10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00) \\
 \bar{\sigma} &= 10.00 \text{ psi}
 \end{aligned}$$

$$\begin{aligned}
 \text{On the other hand, } \bar{\sigma} &= 10.00 \text{ psi} \\
 \bar{\sigma} &= \frac{1}{n} \sum_{i=1}^n \sigma_i \\
 \bar{\sigma} &= \frac{1}{10} (10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00 + 10.00) \\
 \bar{\sigma} &= 10.00 \text{ psi}
 \end{aligned}$$

2. Correction to stresses resulting from movement of base.

Due to the movement of the base it was necessary

to apply a correction to the stresses computed in section 4



above. The manner in which the stresses were corrected is shown below.

|                   | $H_E$  | $D_E$    |
|-------------------|--------|----------|
| Base Pinned       | .513 P | 0        |
| Actual Conditions | Y      | Q        |
| Base on Rollers   | 0      | .00341 P |

Q is the average of the outward deflection at the two bases.

Y is the corrected value of horizontal reaction ( $H_E$ ) due to movement of the bases. The corrected stresses are obtained by multiplying the computed values, as obtained in section 4 above, by  $\frac{Y}{.513 P}$ .

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F. Load Tests and Results

## 1. Deflections

## a. Deflections at C

| <u>Load</u> | <u>Dial Reading</u> |               | <u>Act. Def.</u> | <u>Corr. Def. (Z)</u> | <u>% Diff.</u> |
|-------------|---------------------|---------------|------------------|-----------------------|----------------|
|             | <u>Zero</u>         | <u>Loaded</u> |                  |                       |                |
| 10          | .1085               | .11225        | .00375           | .00339                | 10.6           |
| 20          | .1095               | .1166         | .0071            | .00683                | 3.2            |
| 30          | .1095               | .1212         | .0117            | .01036                | 12.9           |
| 40          | .1090               | .1250         | .0160            | .01443                | 9.8            |
| 50          | .1090               | .1290         | .0200            | .01796                | 11.3           |

## b. Deflection at B or D

| <u>Load</u> | <u>Dial Reading</u> |               | <u>Act. Def.</u> | <u>Corr. Def.</u> | <u>% Diff.</u> |
|-------------|---------------------|---------------|------------------|-------------------|----------------|
|             | <u>Zero</u>         | <u>Loaded</u> |                  |                   |                |
| 10          | .04085              | .04185        | .0010            | .000609           | 64.0           |
| 20          | .0415               | .0434         | .0019            | .00192            | 1.0            |
| 30          | .0549               | .0579         | .0030            | .00288            | 4.2            |
| 40          | .0483               | .0528         | .0045            | .00464            | 3.0            |
| 50          | .0537               | .0478         | .0059            | .0060             | 1.7            |

## c. Sum of the Deflections of the two bases

| <u>Load</u> | <u>Deflection</u> |
|-------------|-------------------|
| 10          | .0009             |
| 20          | .0020             |
| 30          | .0031             |
| 40          | .0054             |
| 50          | .0066             |

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## 2. Stresses

P = 10 lbs.

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|
| Zero        | 0220     | 1878     | 1202     | 0770     | 0220     | 1699     | 2000     | 1340     | 1069     | 1981      |
| Loaded      | 0206     | 1865     | 1217     | 0778     | 0216     | 1686     | 2003     | 1352     | 1060     | 1977      |
| e           | 14       | 13       | 15       | 8        | 4        | 13       | 3        | 12       | 9        | 4         |
| Act. f      | 140      | 130      | 150      | 80       | 40       | 130      | 30       | 120      | 90       | 40        |
| Corr. f     | 140      | 140      | 151.5    | 67.7     | 67.7     | 151.5    | 51.2     | 130      | 130      | 59        |
| % Diff.     | 0        | 7.1      | 1.0      | 18.1     | 40.7     | 14.1     | 41.6     | 7.7      | 32.5     | 32.2      |

P = 20 lbs.

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|
| Zero        | 0220     | 1878     | 1202     | 0770     | 0220     | 1699     | 2000     | 1340     | 1069     | 1981      |
| Loaded      | 0193     | 1850     | 1230     | 0783     | 0209     | 1673     | 2008     | 1362     | 1051     | 1971      |
| e           | 27       | 28       | 28       | 13       | 11       | 26       | 8        | 22       | 18       | 10        |
| Act. f      | 270      | 280      | 280      | 130      | 110      | 260      | 80       | 220      | 180      | 100       |
| Corr. f     | 280      | 280      | 304      | 136      | 136      | 304      | 102      | 260      | 260      | 118       |
| % Diff.     | 3.5      | 0        | 7.8      | 4.4      | 19.1     | 14.5     | 21.6     | 18.2     | 30.8     | 15.2      |

P = 30 lbs.

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|
| Zero        | 0220     | 1878     | 1198     | 0770     | 0220     | 1698     | 1998     | 1330     | 1072     | 1981      |
| Loaded      | 0178     | 1836     | 1241     | 0788     | 0200     | 1660     | 2011     | 1373     | 1041     | 1965      |
| e           | 42       | 42       | 43       | 18       | 20       | 38       | 13       | 43       | 31       | 16        |
| Act. f      | 420      | 420      | 430      | 180      | 200      | 380      | 130      | 430      | 310      | 160       |
| Corr. f     | 419      | 419      | 457      | 204      | 204      | 457      | 155      | 389      | 389      | 178       |
| % Diff.     | .2       | .2       | 5.9      | 11.7     | 1.9      | 16.9     | 16.1     | 10.5     | 20.3     | 10.1      |



P = 40 lbs.

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|
| Zero        | 0219     | 1874     | 1198     | 0768     | 0220     | 1698     | 1998     | 1330     | 1072     | 1981      |
| Loaded      | 0161     | 1821     | 1251     | 0791     | 0193     | 1649     | 2015     | 1381     | 1033     | 1960      |
| e           | 58       | 53       | 53       | 23       | 27       | 49       | 17       | 51       | 39       | 21        |
| Act. f      | 580      | 530      | 530      | 230      | 270      | 490      | 170      | 510      | 390      | 210       |
| Corr. f     | 557      | 557      | 604      | 270      | 270      | 604      | 205      | 517      | 517      | 236       |
| % Diff.     | 4.1      | 4.1      | 12.2     | 14.8     | 0        | 18.8     | 17.0     | 1.3      | 24.6     | 11.0      |

P = 50 lbs.

| <u>Gage</u> | <u>1</u> | <u>2</u> | <u>3</u> | <u>4</u> | <u>5</u> | <u>6</u> | <u>7</u> | <u>8</u> | <u>9</u> | <u>10</u> |
|-------------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|
| Zero        | 0219     | 1874     | 1198     | 0768     | 0220     | 1698     | 1998     | 1330     | 1072     | 1981      |
| Loaded      | 0149     | 1806     | 1263     | 0796     | 0186     | 1634     | 2018     | 1393     | 1023     | 1953      |
| e           | 70       | 68       | 65       | 28       | 34       | 64       | 20       | 63       | 49       | 28        |
| Act. f      | 700      | 680      | 650      | 280      | 340      | 640      | 200      | 630      | 490      | 280       |
| Corr. f     | 696      | 696      | 756      | 337      | 337      | 756      | 256      | 645      | 645      | 295       |
| % Diff.     | .6       | 2.3      | 14.0     | 16.9     | .9       | 15.3     | 21.3     | 2.3      | 24.9     | 18.6      |

|   |   |   |   |   |   |   |   |   |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |    |     |
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| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 | 51 | 52 | 53 | 54 | 55 | 56 | 57 | 58 | 59 | 60 | 61 | 62 | 63 | 64 | 65 | 66 | 67 | 68 | 69 | 70 | 71 | 72 | 73 | 74 | 75 | 76 | 77 | 78 | 79 | 80 | 81 | 82 | 83 | 84 | 85 | 86 | 87 | 88 | 89 | 90 | 91 | 92 | 93 | 94 | 95 | 96 | 97 | 98 | 99 | 100 |
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| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 | 51 | 52 | 53 | 54 | 55 | 56 | 57 | 58 | 59 | 60 | 61 | 62 | 63 | 64 | 65 | 66 | 67 | 68 | 69 | 70 | 71 | 72 | 73 | 74 | 75 | 76 | 77 | 78 | 79 | 80 | 81 | 82 | 83 | 84 | 85 | 86 | 87 | 88 | 89 | 90 | 91 | 92 | 93 | 94 | 95 | 96 | 97 | 98 | 99 | 100 |
|---|---|---|---|---|---|---|---|---|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|-----|



### 3. Conclusions

In the process of testing our beams and the rigid frame on the horizontal loading device, we came across a method of loading that eliminates the possibility of the beam twisting due to the eccentricity of the load.

It was necessary to have some method for centering the load since any twisting causes errors in the values of the stresses. The way this was done for the rigid frame is indicated below.

Gages #1 and #2 were mounted, one on each side of the loaded flange near the load point. The loading yoke was then adjusted so that the stresses indicated by these gages were as near equal as possible. When these stresses are equal, the yoke is applying the load correctly to the frame. By using this method, the percentage error for all strain gages was less.

Although the differences between the observed and calculated stresses and deflections for the rigid frame were greater than for beam number 11, they were still considered satisfactory. There were many more sources of error in the construction of a rigid frame. Possibilities for inaccuracies were introduced in the fabrication of other than a straight model, in splicing, and in the construction of the base detail. The base detail, in particular, introduced complications. It should be noted that corrections to both the deflections and stresses had to be made to compensate for horizontal movement of the base, which was originally designed for no movement.

[illegible]

In general, the authors felt that the results indicate the overall soundness of the techniques and methods used.



## VI. Discussion

In writing this thesis, we have attempted to break it down into sections so that each division was a subject in itself. This method allowed us to gather the results from each test and to present them along with the material from which they were deduced. Therefore, it will not be necessary for us to mention the results we have already listed. There are, however, several items of a general nature that are of interest as an overall result of each method attempted.

The welding of aluminum using eutecrod was very difficult. It took weeks of practice for us to become proficient enough to weld the aluminum without fear of completely melting the parent material. Also, the heating of the aluminum to a high temperature annealed it so that a large furnace for heat treatment would be required to temper it. We, therefore, conclude that eutecrod welding is impractical for building models in the laboratory.

From the tests we have run, we feel that the construction of accurate models by soldering is practical. It is definitely possible to construct models and to obtain reasonable results with close accuracy. The major fault with soldering is that low loads must be applied in order to stay within the required limits of horizontal shear. Although the models constructed were near perfect, the allowable stress was never developed, and it was, therefore, impossible to ascertain the effects of high stresses.

high stresses.

and it was, therefore, impossible to associate the effects of  
were near perfect, the aluminum stress was a very low value,  
limits of horizontal error. Also, the stress was a very low  
low loads were applied. The stress was a very low value,  
with close accuracy. The stress was a very low value,  
possible to obtain a stress of about 1000 psi. The stress  
of accurate results. The stress was a very low value,  
from the stress of about 1000 psi. The stress was a very low value,

which, therefore, was a very low value.

fore, the stress was a very low value.

heat stress was a very low value.

high stress was a very low value.

part of the stress was a very low value.

to the stress was a very low value.

It was, therefore, a very low value.

in the stress was a very low value.

interior stress was a very low value.

and, therefore, was a very low value.

in the stress was a very low value.

which, therefore, was a very low value.

one of the stress was a very low value.

low stress was a very low value.

and, therefore, was a very low value.

The results from the tests run on the beams constructed with steel and silver solder were not satisfactory. The beams obtained from the furnace method seemed absolutely perfect. We have no explanation for the poor results obtained, other than perhaps that the joint was not perfectly soldered although it appeared to be so. In view of the high loads the horizontal loading frame is capable of handling, we feel that the investigation of steel should be continued. It is definitely the feeling of the authors that a small amount of work with the steel method would produce very satisfying results.





## VII. Conclusions

A. Aluminum soldering, using alladin rod to form the joint between model components, which are assembled in accordance with jigging method number 3 modified, is suitable for model construction.

B. Aluminum welding as a method of joining model components is not feasible because of the amount of time required to become proficient in welding, and because of the uncontrollable warping and distortion attendant with it.

C. Furnace brazing aluminum, using eutecrod as the filler material, is not possible.

D. With further work and development, a method of silver soldering steel to fabricate models suitable for high stresses could be evolved.







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